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TRIANGULATION COMPUTATIONS

MARSHALL ISLANDS

ENIWETOK ATOLL

1948

*Walter A. ...
Cross-section of ...
April 1954*

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TRIANGULATION

A scheme of first-order triangulation composed of check figures was executed along the eastern side of the atoll from a first-order base line on Runit Island. This scheme extends northward to Engebi Island and southward to Aniyaanii Island, and was executed for the purpose of coordinating local surveys on the activated islands and to establish distances and azimuths between certain installations.

All observations were made at night, and standard procedure was followed throughout. The maximum triangle closure for the entire scheme was 02.41 seconds and the average 01.01 seconds. The maximum triangle closure in the base expansion figure was 01.10 seconds and the average 00.55 seconds.

With the exception of station REEF PHOTO TOWER, which could not be occupied because of excessive vibration, all stations were occupied. Traverse ties were made to the Zero and/or Photo towers on Engebi, Aomon, Runit, and Aniyaanii Islands.

In order to coordinate the triangulation of the U.S.S. BOWDITCH with the new survey, stations NORTH BASE USN, 1944 and SAND USN, 1944 were incorporated into the scheme.

BASE LINE

A first-order base line was measured on Runit Island between stations NORTH BASE USN, 1944 and newly established station RUNIT. The configuration of the island necessitated the adoption of a broken base consisting of four sections of varying lengths. First-order invar tapes were used, and standard procedure followed throughout.

The computed probable error of the total measurement is 1 part in 2,100,000.

This base line is shorter than the second-order base line measured on the same island by the U.S.S. BOWDITCH in 1944 due to the fact that a considerable portion of the sand spit extending off the south end of the island has washed away.

The base expansion figure was developed through station CORAL, a newly established station constructed in the same general area as station REEF USN, 1944, but not identical with that station. This is the most advantageous position at which the construction of a station suitable for observations was feasible.

The base expansion figure does not meet the specifications for first-order triangulation in that the R_1 for the figure is larger than is generally allowed on first order triangulation. This is the only respect in which the triangulation fails to meet first-order specifications, and is largely compensated by the limited extent of the scheme.

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DATUM

No astronomic observations were made. The datum adopted was that established by the survey of the U.S.S. BOWDITCH in 1944, which is considered to meet all requirements of the present survey.

The probable errors of the elements of the datum as determined by the U.S.S. BOWDITCH are as follows:

Latitude	0.11
Longitude	0.11
Azimuth	0.11

Although the principal azimuth observations were made from station NORTH BASE USN, 1944 to station SAND BASE USN, 1944, a line which no longer exists, an examination of the correction obtained for the angle in the adjustment of the BOWDITCH triangulation shows that but little accuracy is lost by using the azimuth of the line NORTH BASE USN, 1944 to SAND USN, 1944. It was therefore considered that a reobservation for the purposes of the present survey was not justified.

In the computation of the new first-order scheme, the latitude and longitude of station NORTH BASE USN, 1944 and the forward azimuth of the line NORTH BASE USN, 1944 to SAND USN, 1944 have been held, and together with the elements of the Clarke spheroid of 1866 determine the datum used.

RECOMPUTATION OF USN STATIONS.

Since the introduction of a new base line into the scheme changes the length obtained for the line NORTH BASE USN, 1944 to SAND USN, 1944, a recomputation was made for stations of the BOWDITCH survey to the southward of the first-order scheme in order to make them consistent with the new scheme and available for use on the present project. This recomputation was made from the line NORTH BASE USN, 1944 -- SAND USN, 1944. The adjusted angles of the BOWDITCH surveys were used.

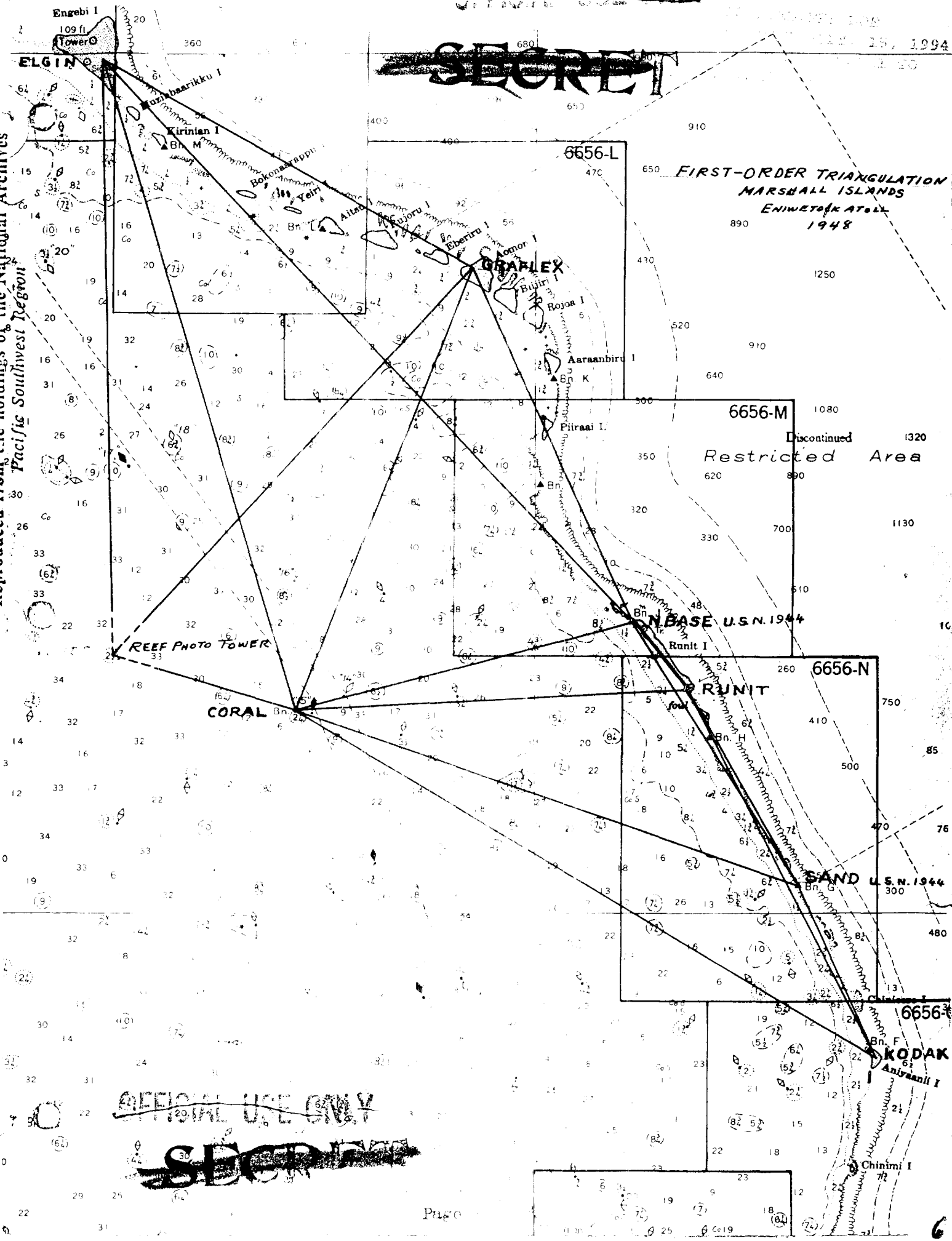
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FIRST-ORDER TRIANGULATION
MARSHALL ISLANDS
ENIWETOK ATOLL
1948

6656-M

Discontinued
Restricted Area

BASE U.S.N. 1944

6656-N

RUNIT

SAND U.S.N. 1944

6656-F

KODAK

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 LETTER DATED JULY, 15, 1994
 FROM ANTON SINISGALLI TO
 DIANE S. NIXON

DEPARTMENT OF COMMERCE
 U.S. COAST AND GEODETIC SURVEY
 FORM 56-B
 Ed. April 1940

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FIELD COMPUTATION
GEOGRAPHIC POSITIONS

ENIWETOK ASTRONOMIC DATUM, 1944

Accession No. of Computation: _____

Locality ENIWETOK ATOLL

FIRST-order Triangulation. State MARSHALL ISLANDS

STATION	LATITUDE AND LONGITUDE	SECONDS IN METERS	AZIMUTH	BACK AZIMUTH	TO STATION	DISTANCE		
						LOGARITHM METERS	METERS	FEET
NORTH BASE, U.S.N., 1944	11 33 23.265		327 56 52.40		SAND, U.S.N., 1944			
	162 21 07.890							
SAND, U.S.N., 1944	11 30 18.781		147 57 15.78	227 56 52.40	NORTH BASE, U.S.N., 1944	3 824 1990	2180.23	47, 711.0
	162 23 06.873							
CORAL	11 32 40.255		253 51 25.04	75 52 42.88	NORTH BASE, U.S.N., 1944	3 824 1990	2180.23	47, 711.0
			247 02 52.95	109 54 14.15	SAND, U.S.N., 1944	3 824 1990	2180.23	47, 711.0
	11 32 40.255		253 51 25.04	75 52 42.88	CORAL	3 824 1990	2180.23	47, 711.0
	162 23 06.873		247 02 52.95	109 54 14.15	CORAL	3 824 1990	2180.23	47, 711.0
KUDAK	11 28 22.674		120 56 28.70	300 35 07.16	CORAL	3 824 1990	2180.23	47, 711.0
	162 22 08.135		34 00 12.79	333 39 49.43	RUWIT	3 708 2891	8096.35	26 562.8
GRAPLES	11 37 12.644		111 21 22.15	313 57 31.57	NORTH BASE, U.S.N., 1944	3 824 1990	2180.23	47, 711.0
	162 23 06.873		111 21 22.15	313 57 31.57	CORAL	3 824 1990	2180.23	47, 711.0
	11 37 12.644		111 21 22.15	313 57 31.57	GRAPLES	3 824 1990	2180.23	47, 711.0
	162 23 06.873		111 21 22.15	313 57 31.57	GRAPLES	3 824 1990	2180.23	47, 711.0
REEF PHOTO	11 32 34.13		180 07 09.00	00 07 04.24	ELGIN	4 043 7715	12409.99	40, 115.1
	162 17 54.051		285 37 19.13	105 37 47.12	CORAL	3 637 2242	4377.49	7, 135.2
			266 31 08.64	86 32 25.14	RUWIT ZERO TOWER	7 064 2928	11575.59	38, 043.2
			223 24 17.41	43 2 09.14	ANTON ZERO TOWER	4 055 4317	11361.40	32, 276.9
			176 16 49.13	356 1 43.40	ENGBI ZERO TOWER	4 123 5877	13291.92	43, 608.6

No check on this position. Abbreviations used: d.=described; m.=marked; n.=not; r.=recovered; l.=lost; p.=probably. (Examples: n. d.=not described; p. l.=probably lost.)

* Derived by inverse Position Computation Page 5

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Station... CORAL

State... Marshall Islands (Eniwetok Atoll)

Chief of party... R.L. Pfeiffer

Date... 2-17-41

Computed by... M.G.

Observer... G.R. Strode

Instrument... J.F.

Checked by... JRS

OBSERVED STATION	Observed direction	Instrument	Sea level reduction	Corrected direction with zero initial	Adjusted direction*
ELGIN	0 00 00.00			0 00 00.00	
GRAFLEX	31 14 18.40				
N.BASE U.S.N. 144	30 17 17.70				
RUNIT	17 43 17.00				
SAND U.S.N. 194 4	25 56 25.40				
KODAK	17 46 10.40				
REEF PHOTO TOWER	33 33 19.10				

No eccentricity of light as determined at this station.

Observations made for 40 each cond station.

Recorded in volumes 1 and 2.

NOTE: No reference made to this station.

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Station ELGIN

State Marshall Is. (Hawetok Atoll)

Chief of party R. L. Pfau

Date 2-14-48

Computed by G.R.S.

Observer G.R.S.

Instrument 4-33

Checked by R.L.P.

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OBSERVED STATION	Observed direction with zero initial	Adjusted direction*
GRAFLEX	00 00 00.00	
N. BASE U.S.N. 1944	02 20 02.00	
CORAL	01 00 01.00	
REEF PHOTO TOWER	01 00 01.00	
ENGEEI ZERO TOWER	01 00 01.00	

No eccentricity of lights or instrument at this station.
Observations made from a 17 foot steel tower.
Observations recorded in volume 3.
NOTE: No reference marks were established at this station.

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DEPARTMENT OF COMMERCE
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 Form 24A
 Rev. Oct., 1932

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LIST OF DIRECTIONS

Station GRAFLEX State Marshall Islands (eniwetok idoll)
 Chief of party R.L. Pflau Date 2-18-41 Observed by R.G.
 Observer G.R. Strode Instrument S.T. Checked by J.H.F.

OBSERVED STATION	Observed direction	Instrument	Sea level reduction	Corrected direction with zero initial	Adjusted direction*
N. BASE U.S.N. 1941	00 00 00.00			0 00 00.00	
CORAL	45 19 15.74				
REEF PHOTO TOWER	07 49 24.02			27 49 24.02	
ELGIN	24 38 07.17				
<i>Amon 200 tower</i> <i>Hor. Dist. 25,050 ft</i> <i>Amon Photo tower Ebr</i> <i>Hor. Dist. 25,050 ft</i>					

No eccentricity of light in statement of observations.
 Observations made from 30 ft steel tower.
 Recorded in volume 3.
 NOTE: No reference made to instrument used.

* Observations recorded in the log book of the vessel.
 * Record of measurements of the instrument used.

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Form 24A
Rev. Oct., 1932

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Station RODAK

Sal. Marsial Islands

Chief of party R.L. Pfeil

Date 2-13-45

Computed by M.G.

Observer G.R. Strode

Instrument G375

Checked by YRS

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OBSERVED STATION

Observed direction

Point to which

observed

Corrected direction with zero refraction

Adjusted direction*

R/L

UNIT

AND USN 19 44

00 00.00

00 00.00

R.M.No.1 NE Hor.

Dist. 57.39 ft

17.495 meters

00 00 00

Aniyaanii photo tower

SE Hor. Dist. 21.425 ft

00 00 00

R.M.No.2 SSE Hor.

Dist. 110.813 ft

33.756 meters

00 00 00

No eccentricity of yard or instrument at this station.

Observations made from a 30 ft. high tower.

Recorded in volume 1.

*Distance to the Aniyaanii photo tower was measured with a standard steel tape G7661 with tension of 50 lbs.

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LIST OF DIRECTIONS

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 24A
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Station N. BASE U.S.N. 1944

Date 1-16-43

Chief of party R.L. Pratt

Computed by M.C.

Observer G.R. Strode

Instrument G. 31

Checked by J.H.P.

U. S. GOVERNMENT PRINTING OFFICE: 1933 11-2643

OBSERVED STATION	Observed direction	Great circle	Sea level reduction	Corrected direction with zero initial	Adjusted direction*
RUNIT	0 00 00.00			0 00 00.00	
SAND U.S.N. 1944	09 09 34.0				
CORAL	00 22 55.0				
EIGIN	07 48 44.0				
GR FLEX	09 36 34.0				
RUNIT PHOTO Tower Hor. Dist. 40.16420	259 54 00.0				
RUNIT ZERO TOWER Hor. Dist. 199.61500	259 11 31.0				

Noncentricity of light or instrument at this station.
 Observations made from photo tower.
 Recorded in volume 2.
 NOTE: No reference made to this station.

* Record of observations in Volume 2
 † Record of measurements in Volume 2

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LIST OF DIRECTIONS

Station... RUNIT

Local... Marshall Is. (Palmyra Atoll)

Chief of party... R. L. Pfau

Date... 7-16-33

Computed by... G.R.S.

Observer... G.R.S. & J.C.H.

Instrument... GRT-100

Checked by...

11-9503

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OBSERVED STATION	Observed direction	Adjusted direction*
SAND U.S.N. 1944	000 00 00	00 00 00.00
KODAK	00 00 00.00	
CORAL	00 00 00.00	
N. BASE U.S.N. 1944	00 00 00.00	
Reference mark No. 1		
41.075 feet, 12.520 meters	00 00 00.00	
Reference mark No. 2		
48.062 feet, 14.650 meters	00 00 00.00	

No eccentricity of light of instrument at this station.
Observations made from a level platform.
Observations recorded by the observer in the field and in
Horizontal angles volume 1.
NOTE: Reference mark elevations were obtained by use of instrument H-374.

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 Rev. Oct., 1932

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LIST OF DIRECTIONS

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Station SAND U.S.N. 1944

State Marshall Islands (Eniwetok Atoll)

Chief of party R.L. Pfeiffer

Date 2-14-44

Computed by M.G.

Observer G.R. Strode

Instrument G.M.

Checked by JRS

OBSERVED STATION	Observed direction	Sea level reduction	Corrected direction with zero initial	Adjusted direction*
KODAK	00 00 00.00		0 00 00.00	
CORAL	00 00 00.00			
N BASE U.S.N. 1944	00 00 00.00			
RUNIT	00 00 00.00			

No eccentricity of limb of instrument.
 Observations made from the wood tower.
 Recorded in volume 1.
 NOTE: No reference marks at this station.

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DUPLICATION CENTER

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 382
Ed. June 1929

Eccentric Light at Station
REEF PHOTO TOWER

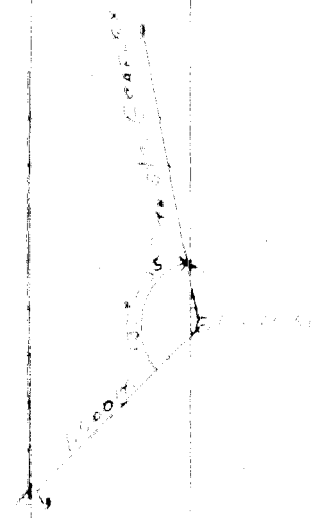
July 21, 1934
Original of 1st 1st 34
S. G. 2nd 2nd 2nd

16-10440

d = 1.500 meters

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STATION	DATE	TIME	LOGARITHM OF REDUCTION IN SECONDS	REDUCTION IN SECONDS
Center				*
Graflex	131 01	4 43 10	4.06 21 77 35	1.26187 +19.28



Ecc. Light at station Reef Photo Tower
is observed in form of the circle ONLY
light is not observed from other
stations.

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ABSTRACT OF DIRECTIONS

Station 302A

Computed by [unclear]

Date 2-17-48

Observer [unclear]

Checked by [unclear]

Inst. No. 6335

POSITION NO.	STATIONS OBSERVED					REFL PHOTO TOWER	
	11618	11619	11620	11621	11622	KODAK	PHOTO
	00° 00'	00° 00'	00° 00'	00° 00'	00° 00'	137.4	302 31
1	0.00	000	000	000	000	42.4	18.4
2	0.00	000	000	000	000	40.1	18.2
3	0.00	000	000	000	000	39.7	19.9
4	0.00	000	000	000	000	40.5	18.6
5	0.00	000	000	000	000	37.6 (35.9) (36.4)R	16.3 (15.3)R
6	0.00	000	000	000	000	40.1	17.6
7	0.00	000	000	000	000	40.9	17.3
8	0.00	000	000	000	000	39.7	17.2
9	0.00	000	000	000	000	43.5	20.4
10	0.00	000	000	000	000	38.5	21.6
11	0.00	000	000	000	000	42.8	20.4
12	0.00	000	000	000	000	41.2	21.2
13	0.00	000	000	000	000	40.9	19.9
14	0.00	000	000	000	000	42.8	22.0
15	0.00	000	000	000	000	40.8	22.3
16	0.00	000	000	000	000	37.9	21.2
Sum,		17.00	17.00	17.00	17.00	113	152.4
Mean,		1.0625	1.0625	1.0625	1.0625	10.94	19.52
Cor. for cor.							
Direction,							

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ABSTRACT OF DIRECTIONS

Station *ELGIN* (Output) *1000* Date *2-19-48*

Observer *G. R. Siroda* (Checked) *1/27/48* Inst. No. *6335*

U. S. GOVERNMENT PRINTING OFFICE: 1935

11-4689

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POSITION No.	STATION DESIGNATION	INITIAL	0° 00'	01'	02'	03'	04'	05'	06'	07'	08'	09'	10'	11'	12'	13'	14'	15'	16'	Sum,	Mean,	Cor. for ecc.,	Direction,
	<i>GRANLEX</i>	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
1	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
2	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
3	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
4	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
5	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
6	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
7	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
8	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
9	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
10	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
11	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
12	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
13	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
14	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
15	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
16	<i>1000</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
Sum,		<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
Mean,		<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>	<i>00</i>				
Cor. for ecc.,																							
Direction,																							

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ABSTRACT OF DIRECTIONS

Station *GRAFFLE* (Computed by *...*) Date *2-18-48*

Observer *G. K. S. ...*

Checked by *...*

Inst. No. *6335*

U. S. GOVERNMENT PRINTING OFFICE: 1941

11-4889

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POSITION NO.	STATIONS OBSERVED				N. BASE PHOTO TOWER
	N. BASE U.S.N. 1944 (INITIALS 0° 0' 0")	
1	0.00	120
2	0.00	
3	0.00	
4	0.00	
5	0.00	075
6	0.00	
7	0.00	
8	0.00	
9	0.00	158
10	0.00	
11	0.00	
12	0.00	
13	0.00	
14	0.00	
15	0.00	
16	0.00	
Sum,		053
Mean,		11.8
Cor. for ecc.					
Direction,					

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INITIALS

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ABSTRACT OF DIRECTIONS

Station *KODAI* Computed by *...*

Date *6/25/48*

Observer *G. R. STROUD* Observed by *...*

Inst. No. *6352*

U. S. GOVERNMENT PRINTING OFFICE: 1931

11-4689

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POSITION NO.	STATE	INSTRUMENT
	<i>CORAL</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
	(INITIAL)						
	0° 00'						
1	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
2	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
3	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
4	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
5	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
6	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
7	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
8	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
9	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
10	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
11	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
12	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
13	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
14	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
15	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
16	0.00	<i>48.1</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
Sum.		<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
Mean,		<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
Cor. for cor.,		<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>
Direction,		<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>	<i>...</i>

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ABSTRACT OF DIRECTIONS

~~CONFIDENTIAL~~

Station H44-46 Computed by [Signature] Date 2-13-48
Observer G. C. SPANGLER Checked by [Signature] Inst. No. 6335

11-4688

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POSITION NO.	STATION OBSERVED							
	SAME	1	2	3	4	5	6	7
	15N 27W							
	(INITIALS)							
	0° 10'							
1	0.00							
2	0.00							
3	0.00							
4	0.00							
5	0.00							
6	0.00							
7	0.00							
8	0.00							
9	0.00							
10	0.00							
11	0.00							
12	0.00							
13	0.00							
14	0.00							
15	0.00							
16	0.00							
Sum,								
Mean,								
Cor. for ecc.,								
Direction,								

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~~CONFIDENTIAL~~

SECRET

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
FORM 470
Ed. Oct., 1932

~~SECRET~~

ABSTRACT OF DIRECTIONS

State *Washington*

Station *N. BASE GSN 194*

Computed by *MG*

Date *2-6-48*

Observer *G. R. Strode*

Checked by *GRS*

Inst. No. *6335*

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Pacific Southwest Region

DO NOT WRITE IN THIS MARGIN

POSITION NO.	(INITIAL) 0° 00"	STATION	STATION	STATION	STATION	STATION	STATION	STATION
		<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
1	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
2	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
3	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
4	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
5	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
6	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
7	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
8	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
9	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
10	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
11	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
12	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
13	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
14	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
15	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
16	0.00	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
Sum.		<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
Mean.		<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>	<i>25.2</i>
Cor. for cor.								
Direction.								

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ABSTRACT OF DIRECTIONS

Sta. MARK 5422 (3-44) (100-2470) (A1021)

Station RUNN

Computed by ...

Date 2-10-48

Observer G.L. Strick

Checked by ...

Inst. No. 6335

POSITION NO.	STATION	BSR	LC					
	<u>SAND</u>	<u>11145</u>	<u>108.02</u>	<u>3.44</u>				
	<u>USN 1944</u>			<u>25.1</u>				
	(INITIAL)							
	0° 00'	<u>58</u>	<u>19</u>	<u>35</u>				
1	0.00	<u>20.4</u>	<u>108.0</u>	<u>10.0</u>				
2	0.00	<u>21.6</u>	<u>108.0</u>	<u>10.0</u>				
3	0.00	<u>22.8</u>	<u>108.0</u>	<u>10.0</u>				
4	0.00	<u>24.0</u>	<u>108.0</u>	<u>10.0</u>				
5	0.00	<u>25.2</u>	<u>108.0</u>	<u>10.0</u>				
6	0.00	<u>26.4</u>	<u>108.0</u>	<u>10.0</u>				
7	0.00	<u>27.6</u>	<u>108.0</u>	<u>10.0</u>				
8	0.00	<u>28.8</u>	<u>108.0</u>	<u>10.0</u>				
9	0.00	<u>30.0</u>	<u>108.0</u>	<u>10.0</u>				
10	0.00	<u>31.2</u>	<u>108.0</u>	<u>10.0</u>				
11	0.00	<u>32.4</u>	<u>108.0</u>	<u>10.0</u>				
12	0.00	<u>33.6</u>	<u>108.0</u>	<u>10.0</u>				
13	0.00	<u>34.8</u>	<u>108.0</u>	<u>10.0</u>				
14	0.00	<u>36.0</u>	<u>108.0</u>	<u>10.0</u>				
15	0.00	<u>37.2</u>	<u>108.0</u>	<u>10.0</u>				
16	0.00	<u>38.4</u>	<u>108.0</u>	<u>10.0</u>				
Sum,		<u>1024</u>	<u>108.0</u>	<u>160.0</u>				
Mean,		<u>26.6</u>	<u>108.0</u>	<u>10.0</u>				
Cor. for cur.,								
Direction,								

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U. S. COAST AND GEODETIC SURVEY
Form 470
Ed. Oct. 1944

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N 171 05
S 175 20
ABSTRACT OF DIRECTIONS

Station SAN LEON ...

Station SAN LEON

Computed by ...

Date 2-11-48

Observer B. H. Stone

Checked by ...

Inst. No. 6735

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POSITION NO.	STATE NO.	INITIAL	0° 00'	138° 00'	179° 00'	179° 00'	0	0	0	0	0
1			0.00								
2			0.00								
3			0.00								
4			0.00								
5			0.00								
6			0.00								
7			0.00								
8			0.00								
9			0.00								
10			0.00								
11			0.00								
12			0.00								
13			0.00								
14			0.00								
15			0.00								
16			0.00								
Sum,				155	245	200					
Mean,				1045	1525	1250					
Cor. for con.,											
Direction,											

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DEPARTMENT OF COMMERCE
U.S. COAST AND GEODETIC SURVEY
Form 25
FEBRUARY 1949

COMPUTATION OF TRIANGLES

Station: *Marshall Is. Area - Eniwetok Atoll*

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Pacific Southwest Region

STATION	DATE	TIME	HEAVY	HEAVY	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					2591.2689m	3 413 6298 ✓
1 Coral	15.48	98.00	10.02	0.02	0950	0.564 9132 ✓
2 N. Base, etc.	112.24	103.00	59.77	0.01	55.76	9 966 3991 ✓
3 Reef	81.28	107.00	54.74	0.22	54.74	9 896 2271 ✓
1-3						3 944 9421 ✓
1-2						3 874 7701 ✓
			0.05		00.00	
2-3						3.874 7701 ✓
1 Sand, etc.	08.55	110.00	11.69	0.04	11.69	0.202 1920 ✓
2 Coral	04.41	110.00	27.87	0.04	27.87	9 747 8359 ✓
3 N. Base, etc.	107.05	110.00	20.44	0.04	20.44	9 980 3895 ✓
1-3						3.824 7980 ✓
1-2						4 057 3516 ✓
			0.12		00.00	
2-3						3.944 9421 ✓
1 Sand, etc.	40.08	110.00	11.13	0.03	11.13	0.173 2968 ✓
2 Coral	10.10	110.00	28.33	0.03	28.33	9 495 1219 ✓
3 Reef	119.25	110.00	10.54	0.13	10.54	9 939 1108 ✓
1-3						3.613 3608 ✓
1-2						4.057 3497 ✓
			0.08		00.00	
2-3						3.413 6298 ✓
1 Sand, etc.	00.16	110.00	19.55	0.00	19.55	1.245 7478 ✓
2 N. Base, etc.	00.08	110.00	24.92	0.00	24.92	8 953 9146 ✓
3 Reef	121.00	110.00	05.53	0.01	05.53	9 165 3757 ✓
1-3						3.613 2922 ✓
1-2						3.824 7533 ✓
			0.01		00.00	

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 25
Ed. Nov. 1946

COMPUTATION OF TRIANGLES

State: _____

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Pacific Southwest Region

STATION	OBSERVED ANGLE	CORR'N	SPHER'L ANGLE	SPHER'L EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3						3.944 9421 ✓
1 Kodak	33 03 44.89	-0.80	44.09	0.05	44.04 ✓	0.263 1657 ✓
2 Coral	30 05 33.60	-0.80	32.80	0.05	32.75 ✓	9.700 1813 ✓
3 Runit	116 50 44.08	-0.81	43.27	0.06	43.21 ✓	9.950 4762 ✓
1-3						3.908 2891 ✓
1-2						4.158 5840 ✓
		02.57 -2.41		0.16	00.00	
2-3						4.057 3497 ✓
1 Kodak	35 55 36.07	-0.61	35.46	0.03	35.43 ✓	0.231 5490 ✓
2 Coral	11 52 14.87	-0.61	14.26	0.03	14.23 ✓	9.313 2373 ✓
3 Sand, U.S.N.	132 12 10.97	-0.60	10.37	0.03	10.34 ✓	9.869 6829 ✓
1-3						3.602 1320 ✓
1-2						4.158 5826 ✓
		01.91 -1.82			00.00	
2-3						3.613 3608 ✓
1 Kodak	02 51 51.18	-0.17	51.01	0.00	51.01 ✓	1.301 3048 ✓
2 Runit	02 47 26.84	-0.17	26.67	0.00	26.67 ✓	8.687 4452 ✓
3 Sand, U.S.N.	174 20 42.50	-0.17	42.33	0.01	42.32 ✓	8.993 3778 ✓
1-3						3.602 0928 ✓
1-2						3.908 2634 ✓
		00.52 -0.51			00.00	
2-3						
1						
2						
3						
1-3						
1-2						

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DEPARTMENT OF COMMERCE
 U. S. COAST AND GEODETIC SURVEY
 Form 25
 Ed. Nov. 1946

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COMPUTATION OF TRIANGLES

State: *Marshall Islands ~ Eniwetok Atoll*

	STATION	OBSERVED ANGLE	CORRN	SPHER'L ANGLE	SPHER'L EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
	2-3						3.874 7701 ✓
	1 <i>Graflex</i>	45 59 53.24 ✓	-0.43	52.81	0.05	52.76 ✓	0.143 0806 ✓
	2 <i>N. Base, USN</i>	80 21 39.08 ✓	-0.42	38.66	0.06	38.60 ✓	9.993 8247 ✓
	3 <i>Coral</i>	53 38 29.12 ✓	-0.43	28.69	0.05	28.64 ✓	9.905 9691 ✓
	1-3						4.011 6754 ✓
	1-2						3.923 8198 ✓
			01.44	-1.28		00.00	
	2-3						4.011 6754 ✓
	1 <i>Elgin</i>	46 07 16.82 ✓	+0.16	16.98	0.08	16.90 ✓	0.142 1794 ✓
	2 <i>Graflex</i>	95 38 14.34 ✓	+0.16	14.50	0.07	14.43 ✓	9.997 8945 ✓
	3 <i>Coral</i>	38 14 28.59 ✓	+0.16	28.75	0.08	28.67 ✓	9.791 6729 ✓
	1-3						4.151 7493 ✓
	1-2						3.945 5277 ✓
			59.75	+0.48		00.00	
	2-3						3.874 7701 ✓
	1 <i>Elgin</i>	27 26 12.84 ✓	+0.34	13.18	0.09	13.09 ✓	0.336 5134 ✓
	2 <i>N. Base, USN</i>	60 40 48.70 ✓	+0.34	49.04	0.09	48.95 ✓	9.940 4670 ✓
	3 <i>Coral</i>	91 52 57.71 ✓	+0.34	58.05	0.09	57.96 ✓	9.999 7655 ✓
	1-3						4.151 7505 ✓
	1-2						4.211 0490 ✓
			59.25	+1.02		00.00	
	2-3						4.211 0490 ✓
	1 <i>Graflex</i>	141 38 07.58 ✓	-0.66	06.98	0.04	06.94 ✓	0.207 1425 ✓
	2 <i>N. Base, USN</i>	19 40 50.38 ✓	-0.66	49.77	0.04	49.73 ✓	9.527 3392 ✓
	3 <i>Elgin</i>	18 41 03.98 ✓	-0.66	03.37	0.04	03.33 ✓	9.505 6285 ✓
	1-3						3.945 5307 ✓
	1-2						3.923 8200 ✓
			01.94	-1.82		00.00	

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 25
Ed. Nov. 1946

COMPUTATION OF TRIANGLES

State: *Marshall Islands - Eniwatok Atoll*

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Pacific Southwest Region

STATION	OBSERVED ANGLE	COR'N	SPHER'L ANGLE	SPHER'L EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3						4.011 6754
1 Reef Photo tower	(62 27 20.26)	0.00	20.26	0.04	20.22	0.052 2464
2 Graflex	21 49 30.78	0.00	30.78	0.04	30.74	9.570 2816
3 Coral	95 43 09.07	0.00	09.07	0.03	09.04	9.997 8328
1-3						3.634 2034
1-2						4.061 7546
		00.11	0.00	0.11	00.00	
2-3						3.945 5277
1 Reef Photo tower	(43 02 55.95)	0.00	55.95	0.08	55.87	0.165 8199
2 Elgin	63 08 20.74	0.00	20.74	0.08	20.66	9.950 4165
3 Graflex	73 48 43.56	0.00	43.56	0.09	43.47	9.982 4306
1-3						4.061 7644
1-2						4.093 7782
		00.25		0.25	00.00	
2-3						4.151 7505
1 Reef Photo tower	(105 30 15.73)	0.00	15.73	0.05	15.68	0.016 0986
2 Elgin	11 01 03.92	0.00	03.92	0.04	03.88	9.466 3751
3 Coral	51 28 40.48	0.00	40.48	0.04	40.44	9.925 9224
1-3						3.634 2242
1-2						4.093 7715
		00.13		0.13	00.00	
2-3						
1						
2						
3						
1-3						
1-2						

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26-Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

α	2	to 3	255 01 25.04	α	3	to 2	75 02 12.88
$2dL$		&	+15 48 09.52	$3dL$			-112 14 55.77
α	2	to 1	270 49 34.56	α	3	to 1	322 47 17.11
$\Delta\alpha$			+ 00 58.14	$\Delta\alpha$			+ 00 10.36
			180 00 00.00				180 00 00.00
α'	1	to 2	90 50 32.70	α'	1	to 3	142 47 27.47
		First Angle of Triangle	51 56 34.76				
			42 47 27.47				
ϕ	11 32 20.30		162 17 10.93	ϕ	11 33 23.265	N Base USA	162 21 09.890
$\Delta\phi$			+ 04 50.690	$\Delta\phi$			+ 00 51.731
ϕ'	11 32 16.080	1 RUNIT	162 22 01.621	ϕ'	11 32 16.080	1 RUNIT	162 22 01.621
Logarithms		Logs		Logarithms		Logs	
s	3.777 7421	(1) + 04.1244	s	3.413 6298	(1) + 67.1835	s	3.413 6298
$\cos \alpha$	8.758 7224	(2) + 0.0404	$\cos \alpha$	9.207 336	(2) + 0.0013	$\cos \alpha$	9.781 5865
B	8.202 4997	Sum + 04.1244	B	8.812 4972	Sum + 67.1835	B	8.507 6678
E	7.322 88		E	7.322 88		E	7.322 88
A	7.779 91		A	7.779 91		A	7.779 91
C	0.116 68	(6)	C	0.117 37	(6) +	C	0.117 37
K	8.606 47	(7)	K	7.107 80	(7) +	K	7.107 80
$(\phi)'$	1.232 7	$-\Delta\phi$ + 04.1742	$(\phi)'$	3.654 5	$-\Delta\phi$ + 67.1848	$(\phi)'$	3.654 5
D	1.984 5	$\frac{\Delta\phi}{2}$	D	1.985 1	$\frac{\Delta\phi}{2}$	D	1.985 1
(3)	3.217 2		(3)	5.639 6		(3)	5.639 6
$-h$	0.616 4		$-h$	1.827 3		$-h$	1.827 3
$s^2 \sin^2 \alpha$	7.889 8		$s^2 \sin^2 \alpha$	6.390 4		$s^2 \sin^2 \alpha$	6.390 4
E	5.663 5	Arc-sin corr.	E	5.663 6	Arc-sin corr.	E	5.663 6
(4)	4.169 7	for s - 1 $(\Delta\lambda)^2$ 7.390	(4)	3.881 3	for s - 0 $(\Delta\lambda)^2$ 5.141	(4)	3.881 3
		for $\Delta\lambda$ + 1 F 7.575			for $\Delta\lambda$ + 0 F 7.135		
Total	0	(8) 4.965	Total	0	(8) 2.276	Total	0
		$-\Delta\lambda$ -290.690			$-\Delta\lambda$ -51.7309		

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α	2	to 8	270	49	34.56	α	8	to 2	90	50	32.70
2^dL		&	+30	05	32.80	3^dL		&	-116	50	43.27
α	2	to 1	300	55	07.36	α	8	to 1	333	59	49.43
$\Delta\alpha$			+ 01		21.34	$\Delta\alpha$			+ 00		23.36
			180	00	00.00				180	00	00.00
α'	1	to 2	120	56	28.70	α'	1	to 8	154	00	12.79
First Angle of Triangle			33	03	44.09	First Angle of Triangle			33	03	44.09
			154	09	12.79				154	09	12.79

ϕ	11	32	20.255	2	CORAL	λ	162	17	10.931	ϕ	11	32	16.080	8	RUNIT	λ	162	22	01.621
$\Delta\phi$			07			$\Delta\lambda$	+ 06		47.804	$\Delta\phi$			- 03			$\Delta\lambda$	+ 01		57.114
ϕ'	11	32	17.242	1	KODAK	λ'	162	23	58.735	ϕ'	11	32	14.242	1	KODAK	λ'	162	23	58.735

Logarithms				Logs				Logarithms				Logs							
s	4.158	5840	(1) + 240.9054	1	9.699	$\frac{1}{2}(\phi+\phi')$	11	32	19.15	s	3.908	2891	(1) + 236.8308	1	9.699	$\frac{1}{2}(\phi+\phi')$	11	32	17.66
+ cos α	9.710	8123	(2) + 0.0796	s ²		Logarithms				+ cos α	9.953	6493	(2) + 0.0066	s ²		Logarithms			
B	8.512	4491	Sum + 240.9847	K	s	4.158	5840	B	8.512	4998	Sum + 236.8314	K	s	3.908	2891				
(1)=h	2.381	8460	(3) + 0.0006	E	sin α	9.933	4352	(1)=h	2.374	4382	(3) + 0.0005	E	sin α	9.644	8814				
s ²	8.512		(4) - 0.0002	(5)	A'	8.507	6484	s ²	7.816	58	(4) - 0.0000	(5)	A'	8.512	4491				
sin ² α	7.866	81	(5)	3	0.477	sec ϕ'	0.008	7642	sin ² α	9.283	18	(5)	3	0.477	sec ϕ'	0.008	7642		
C			(6)	cos ² α	Sum	6.610	7018	C	0.16	65	(6)	cos ² α	Sum	2.068	6093				
(2)=K	8.700	72	(7) +	(6)	Arc-sin corr.			(2)=K	7.811	01	(7) +	(6)	Arc-sin corr.						
(ϕ) ²	4.76	38	$-\Delta\phi$ + 240.9851	(color) E	$-\Delta\lambda$	2.610	4517	(ϕ) ²	4.748	9	$-\Delta\phi$ + 236.8379	(color) E	$-\Delta\lambda$	2.068	6092				
D	1.98	45	$\frac{\Delta\phi}{2}$	$\frac{A^2 \sec^2 \phi}{3}$	5.912	sin $\frac{1}{2}(\phi+\phi')$	9.299	8596	D	1.98	45	$\frac{\Delta\phi}{2}$	$\frac{A^2 \sec^2 \phi}{3}$	5.912	sin $\frac{1}{2}(\phi+\phi')$	9.299	8380		
(3)	6.748	8		sec ² ϕ		sec $\frac{\Delta\phi}{2}$	1	(3)	6.733	4		sec ² ϕ		sec $\frac{\Delta\phi}{2}$	1				
-h	2.38	18		(7)	(approx.)	$-\Delta\alpha$	1.910	3114	-h	2.37	44		(7)	(approx.)	$-\Delta\alpha$	1.368	4473		
s ² sin ² α	8.18	40		do		do	-81.342	s ² sin ² α	7.10	04		do		do	-23.358				
E	5.66	35	Arc-sin corr.	(8)		(8)	0	E	5.66	35	Arc-sin corr.	(8)		(8)	0				
(4)	6.22	93	for s - 3.5	($\Delta\lambda$) ²	7.832	$-\Delta\alpha$	-81.342	(4)	5.13	83	for s - 1.1	($\Delta\lambda$) ²	6.206	$-\Delta\alpha$	-23.358				
			for $\Delta\lambda$ + 2.8	F	7.575						for $\Delta\lambda$ + 0.2	F	7.575						
Total	-0.7		(8)	5.407	$-\Delta\lambda$	-40.7		Total	-0.7		(8)	3.781	$-\Delta\lambda$	-117.1141					

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

α 2 to 3				α 3 to 2			
2 ^d Z &				3 ^d Z &			
α 2 to 1				α 3 to 1			
Δα				Δα			
1 to 2				1 to 3			
162	21	09.890	0	162	23	06.873	0
03	04	284	Δλ +	01	56	983	Δφ
11	30	18.981	162	23	06	873	φ'
Logarithms				Logarithms			
s	3.824	7980	9.699	s	(1)		9.699
cos α	9.928	734	Logarithms	cos α	(2)		Logarithms
B	5		2.824	B	Sum		K
h	2.265	4706	sin α	(1)-h	(3) +		E
*	7.649	60	A	(4)	(5) -		(5)
sin ² α	9.449	68	sec φ'	(5)	3	0.477	sec φ'
C	0.717	37	Sum	(6) +	cos ² α		Sum
(2)=K	7.816	65	Arc-sin corr.	(7) +	(6)		Arc-sin corr.
(3) ²	4.53	09	(olog) E	-Δλ	(8) E		Δλ
D	1.9851		A ² arc ² 1"	3	5.912	sin φ(φ+φ')	D
(3)	6.51	60	sec ² φ	sec Δφ/2	0	(3)	sec ² φ
-h	2.2655		(approx.) -Δα	1.368	9264	-h	(approx.) -Δα
s ² sin ² α	7.0993		do	-23.384		s ² sin ² α	do
E	5.6636		(8)	0		E	(8)
(4)	5.0284		Arc-sin corr. for s	-1	(Δλ) ²	6.204	-Δα
			Arc-sin corr. for Δλ	+0	F	2.576	
Total	-1		(8)	3	780	-Δλ	-116.9830

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α	2	to 3	Pacific Southwest Region	56	52.40	α	8	to 1	147	57	15.78
$2^d Z$		&		+107	05	$3^d Z$			-38	53	11.73
α	2	to 1		75	02	α	8	to 1	109	04	04.85
$\Delta\alpha$				-	00	$\Delta\alpha$			-	01	11.10
				180	00				180	00	00.00
α'	1	to 2		255	01	α'	1	to 8	289	02	52.95
			First Angle of Triangle	34	01						
				289	02						
				0	1						

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ϕ	11	33	23.265	2 N Base, U.S.N.	λ	162	21	09.890	ϕ	11	30	18.981	3 Sand U.S.N.	λ	162	23	06.873
$\Delta\phi$			01 03 010		$\Delta\lambda$			00 08 757	$\Delta\phi$			01 274		$\Delta\lambda$			05 55 942
					λ'	162	17	10.931	ϕ'	11	32	20.255	1 CORAL	λ'	162	17	10.931

Logarithms				Logs	Logarithms				Logs									
s	3.874 7701			9.699	$\phi + \phi'$	11 32 51.76			9.699	$\phi + \phi'$	11 31 17.65							
$\cos \alpha$	9.514 1338				Logarithms					Logarithms								
B	8.512 5007	Sum	+ 63.0099	K	s	3.874 7701			B	8.512 5007	Sum	- 121.2749	K	s	4.057 3516			
$\sin \alpha$	9.985 0186				$\sin \alpha$	9.985 0186			$\sin \alpha$	9.985 0186	(1)-h	2.083 9867	(3)	+	0.0001	E	$\sin \alpha$	9.975 4928
A'	8.509 6677				A'	8.509 6677			A'	8.509 6677	s^2	8.144 70	(4)	+	0.0001	(5)	A'	8.509 6677
$\sec \phi$	0.008 8673				$\sec \phi$	0.008 8673			$\sec \phi$	0.008 8673	$\sin^2 \alpha$	7.950 78	(5)	-		(6)	$\sec \phi$	0.008 8673
Sum	2.578 2229				Sum	2.578 2229			Sum	2.551 3793	$\cos^2 \alpha$	0.715 35	(6)	+		(7)	Sum	2.551 3793
Arc-sin corr.					Arc-sin corr.				Arc-sin corr.		(2)=K	8.781 03	(7)	+		(6)	Arc-sin corr.	
$-\Delta\lambda$	2.378 3239				$-\Delta\lambda$	2.378 3239			$-\Delta\lambda$	2.551 3793	(log) E						$-\Delta\lambda$	2.551 3793
$\sin \frac{1}{2}(\phi + \phi')$	9.301 4290				$\sin \frac{1}{2}(\phi + \phi')$	9.301 4290			$\sin \frac{1}{2}(\phi + \phi')$	9.300 4785	$\frac{\Delta\phi}{2}$						$\sin \frac{1}{2}(\phi + \phi')$	9.300 4785
$\sec \frac{\Delta\phi}{2}$					$\sec \frac{\Delta\phi}{2}$				$\sec \frac{\Delta\phi}{2}$		$\frac{A^2 \sec^2 1''}{3}$	5.912					$\sec \frac{\Delta\phi}{2}$	
(approx.) $-\Delta\alpha$	1.679 7529				(approx.) $-\Delta\alpha$	1.679 7529			(approx.) $-\Delta\alpha$	1.851 8578	$\sec^2 \phi$						(approx.) $-\Delta\alpha$	1.851 8578
do	+ 47.836				do	+ 47.836			do	+ 71.098	(7)						do	+ 71.098
$s^2 \sin^2 \alpha$	7.7196				$s^2 \sin^2 \alpha$	7.7196			$s^2 \sin^2 \alpha$	7.7196								
E	5.6636	Arc-sin corr.			E	5.6636	Arc-sin corr.		E	5.6632	Arc-sin corr.						(8)	
(4) 5.1824		for s - 1	($\Delta\lambda$) ²	7.135	(4) 5.8129		for s - 2	($\Delta\lambda$) ²	7.654		for s - 2	($\Delta\lambda$) ²	7.654				$-\Delta\alpha$	+ 71.098
		for $\Delta\lambda$ + 1	F	7.576			for $\Delta\lambda$ + 2	F	7.567									
		Total	+ 0	(8) 4.711			Total	0	(8) 5.221								$-\Delta\lambda$	+ 355.9423

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

α	2	to 3	75	02	12.88	α	3	to 2	255	01	25.04
$2^d \angle$		&	+80	21	38.66	$3^d \angle$		&	-53	38	28.69
α	2	to 1	155	23	51.54	α	3	to 1	201	22	56.35
$\Delta\alpha$			-	00	23.17	$\Delta\alpha$			+	00	24.82
			180	00	00.00				180	00	00.00
α'	1	to 2	335	23	28.37	α'	1	to 3	21	23	21.17
		First Angle of Triangle	45	59	52.81						
			3	3	19						
$\Delta\phi$			102	21	00.00	$\Delta\phi$			102	02	03.63
			102	19	11.08				102	11	47.36
Logarithms			3.923	8198	(1) - 238.3062	Logs			9.699	$\frac{1}{2}(\phi+\phi')$	11.35 27.41
$\cos \alpha$	9.958	6685	(2) + 0.0064	s^2		Logarithms			4.011	6754	(1) - 311.3174
$\sin \alpha$	9.258	85	(5) -	$\cos \alpha$	9.969	2283	(2) + 0.0023	s^2			Logarithms
$\cos^2 \alpha$	0.717	37	(6) +	$\sin \alpha$	9.561	4255	(3) + 0.0003	$\sin \alpha$	9.561	4255	
$(2)=K$	7.803	86	(7) +	$\sin^2 \alpha$	9.123	61	(5) -	$\cos^2 \alpha$	0.477		
$(\phi)^2$	4.79	00	$-\Delta\phi$	$\cos^2 \alpha$	2.061	9138	$-\Delta\lambda$	$\cos^2 \alpha$	2.092	1482	
D	1.785	1	$\frac{\Delta\phi}{2}$	$(\phi)^2$	4.98	64	$-\Delta\phi$	$(\phi)^2$	4.98	64	
(3)	6.775	1	$\frac{\Delta\phi}{2}$	$(\phi)^2$	4.98	64	$-\Delta\phi$	$(\phi)^2$	4.98	64	
-h	2.395	0	$\frac{\Delta\phi}{2}$	$(\phi)^2$	4.98	64	$-\Delta\phi$	$(\phi)^2$	4.98	64	
$s^2 \sin^2 \alpha$	7.086	5	$\frac{\Delta\phi}{2}$	$(\phi)^2$	4.98	64	$-\Delta\phi$	$(\phi)^2$	4.98	64	
E	5.663	6	$\frac{\Delta\phi}{2}$	$(\phi)^2$	4.98	64	$-\Delta\phi$	$(\phi)^2$	4.98	64	
(4)	5.145	1	$\frac{\Delta\phi}{2}$	$(\phi)^2$	4.98	64	$-\Delta\phi$	$(\phi)^2$	4.98	64	
Total	-1.1			$(\phi)^2$	4.98	64	$-\Delta\phi$	$(\phi)^2$	4.98	64	

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POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

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α	2	to 3	21	23	21.18	α	3	Pacific Southwest Region	201	22	56.35
$2^d \angle$		&	495	38	14.50	$3^d \angle$		&	-38	14	28.75
α	2	to 1	117	01	35.68	α	3	to 1	163	08	27.60
$\Delta\alpha$			-	00	52.36	$\Delta\alpha$			-	00	27.31
			180	00	00.00				180	00	00.00
α'	1	to 2	297	00	43.32	α'	1	to 3	343	08	00.29 ⁺
		First Angle of Triangle	46	07	16.98						
			343	08	00.30						

ϕ	11	31	35.564	2	CORALLEX	λ	162	19	14.568	ϕ	11	32	20.255	3	CORAL	λ	162	17	10.931
$\Delta\phi$						$\Delta\lambda$				$\Delta\phi$						$\Delta\lambda$			

Logarithms				Logarithms			
$\cos \alpha$	9.987 9117	2	0.000 0000	$\cos \alpha$	9.980 9016	(2)	0.000 0000
B	5.512 4970	Sum	0.000 0000	B	8.512 4997	Sum	-441 7351
(1) h	7.346 666	3		(1) h	2.645 1706	(3)	0.000 0000
s^2		4		s^2	8.228 4	4	
$\sin^2 \alpha$		5	0.477	$\sin^2 \alpha$		5	0.477
$\sec \phi$		6		$\sec \phi$		6	
Sum		7		Sum		7	
Arc-sin corr		8		Arc-sin corr		8	
(2) K		9		(2) K	7.175	9	
$(\Delta\lambda)^2$	4.230 9	$-\Delta\phi$	50.7245	$(\Delta\lambda)^2$	5.240 3	$-\Delta\phi$	-771.103
D	1.987 6	$\Delta\phi$		D	1.984 5	$\Delta\phi$	
(3)	6.218 5	2		(3)	7.274 8	2	
$-\Delta\phi$	2.115 5	$\frac{A^2 \text{arc}^2 1''}{3}$	5.912	$-\Delta\phi$		$\frac{A^2 \text{arc}^2 1''}{3}$	5.912
$s^2 \sin^2 \alpha$	7.790 6	$\sec^2 \phi$		$s^2 \sin^2 \alpha$	7.228 4	$\sec^2 \phi$	
E	5.664 3	(7)		E	5.653 5	(7)	
(4)	5.570 4	do	+52.360	(4)	5.527 1	do	+27.307
		(8)	0.000			(8)	0.000
		for s	-1.4			for s	-3.6
		($\Delta\lambda$) ³	7.242			($\Delta\lambda$) ³	6.399
		for $\Delta\lambda$	+1.1			for $\Delta\lambda$	+0.3
		F	7.578			F	7.575
		Total	-0.3			Total	-3.3
		(8)	7.820			(8)	3.974
		$-\Delta\lambda$	+259.4364			$-\Delta\lambda$	+135.7994

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

α 2	to 3	343 08 00.30	α 3	to 2	163 08 27.60
$2\frac{1}{2}L$	&	+17 01 03.92	$3\frac{1}{2}L$	&	-57 28 40.48
α 2	to 1	000 09 04.22	α 3	to 1	105 39 47.12
$\Delta\alpha$		- 00 00.22	$\Delta\alpha$		- 00 27.39
		180 00 00.00			180 00 00.00
		80 09 04.00	α' 1	to 3	285 59 19.73
		103 34 57.75			
		285 39 42.25			
		12 14 55.12	ϕ	32 20 20.00	
		00 00 00.00	$\Delta\phi$	00 3 38.00	
		100 00 00.00	ϕ'	32 09 53.00	Real Photo Tower
Logarithms	Logs	3 687 2242	Logarithms	Logs	3 687 2242
		3 687 2242			3 687 2242
Logarithms		3 687 2242	Logarithms		3 687 2242
		3 687 2242			3 687 2242
$\sin^2 \alpha$	0.477	sec ϕ 1.008 3851	$\sin^2 \alpha$	9.967 13	(5)
C	(6) +	Sum 0.033 6717	C	0.716 68	(6) +
(2)-K	(7) +	Arc-sin corr.	(2)-K	7.952 26	(7) +
$(\delta\phi)^2$	$-\Delta\phi$ +403.8927	$-\Delta\lambda$ 0.033 6714	$(\delta\phi)^2$	3.15 61	$-\Delta\phi$ -37.8401
D	$\frac{\Delta\phi}{2}$	$\frac{(\text{colog}) E}{3}$ 5.912	D	1.9845	$\frac{\Delta\phi}{2}$
(3)		$\sec^2 \phi$	(3)	5.1406	
-h		(approx.) $-\Delta\alpha$ 9.337 2415	-h	1.5781	(7)
$s^2 \sin^2 \alpha$		do +0.217	$s^2 \sin^2 \alpha$	7.2356	do +27.393
E	Arc-sin corr.	(8)	E	5.6635	Arc-sin corr.
(4)	for s -2.8	$-\Delta\alpha$ +0.217	(4)	4.4772	for s -0.3
	for $\Delta\lambda$ +0				$-(\Delta\lambda)^2$ 6.409
					for $\Delta\lambda$ +0.4
					F 7.575
	Total -2.8	$-\Delta\alpha$ +1.0806		Total +0.1	(8) 3.984
					$-\Delta\lambda$ +136.8804

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 27
Ed. April 1945

TRAVERSE
POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

			"			— CHECK COMPUTATION —			"		
α	2A KODAK	to B A CORAL	120	56	28.70	α	3	to 2			
2d L		&	+202	56	19.60	3d L		&			
α	2	to 1	323	52	48.30	α	3	to 1	143	52	48.38
$\Delta\alpha$			+	00	00.08	$\Delta\alpha$			-	00	00.08
			180	00	00.0				180	00	00.0
α	1	to 2	43	52	48.38	α	1	to 3	15	52	48.38
Sum			162	23	59.52	Sum			162	23	58.73
Traverse	330 3208		330	28 18 96		Traverse	330 3208		330	28 18 96	
Sum	0 00 00		Sum	0 00 00		Sum	0 00 00		Sum	0 00 00	
$\Delta\alpha$	9.619 8202	-00.4167	$\Delta\alpha$	9.619 8202	-00.4167	$\Delta\alpha$	9.619 8202	+00.4167	$\Delta\alpha$	9.619 8202	+00.4167
2d term	7.278 684		2d term	7.278 684		2d term	7.278 684		2d term	7.278 684	
h^2	9.5014		h^2	9.5014		h^2	9.5014		h^2	9.5014	
D	1.9821		D	1.9821		D	1.9821		D	1.9821	
3d term	1.0000		3d term	1.0000		3d term	1.0000		3d term	1.0000	
$-\Delta\phi$	+00.563		$-\Delta\phi$	+00.563		$-\Delta\phi$	+00.563		$-\Delta\phi$	+00.563	

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DEPARTMENT OF COMMERCE
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Form 27
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POSITION COMPUTATION, ~~ORDER~~ *FIRST* ~~TRIANGULATION~~ *TRAVERSE*

CHECK COMPUTATIONAL

<i>a</i>	<i>b</i>	<i>c</i>	<i>A</i>	<i>B</i>	<i>C</i>	<i>a</i>	<i>b</i>	<i>c</i>	<i>A</i>	<i>B</i>	<i>C</i>
2	161		322	47	17.11	2	162		328	11	31.70
2	161		290	58	48.81	2	161		290	58	48.81
			+ 00		01.23				+ 00		01.23

Total Error of Traverse

161	23,265 2 V BASE, USN	162	21	09.890	33	20,939 3 RUNIT ZERO Tower	162	21	16.041
		+ 00		02.326			+ 00		06.151
33	20,939 3 RUNIT ZERO Tower	162	21	16.041	11	33 23,265 1 1 BASE USN	162	21	17.577

Logarithms values in seconds

<i>a</i>	<i>b</i>	<i>c</i>	<i>A</i>	<i>B</i>	<i>C</i>	<i>a</i>	<i>b</i>	<i>c</i>	<i>A</i>	<i>B</i>	<i>C</i>
0.266 6000			1.110 2093			0.366 4389	1st term		9.970 2083		
4.207 39			X 509 6676			4.600 39			8.509 6675		
0.717 32			Sine 0.008 8936			9.740 42	Sine		0.008 8946		
5.258 18	2d term + 0.0000		-Δx 0.788 9650	-06.1513		0.717 33			0.788 9649	+06.1513	
0.723 3			Sin(φ+φ')	9.301 7416		5.258 14	2d term	+ 0.0000	Sin(φ+φ')	9.301 7416	
1.985 1			-Δa 0.090 7066	-01.232					-Δa 0.090 7065	+01.232	
2.218 4	3d term + 0.0000										
	-Δφ	+2.3261					3d term	+			
									-Δφ	-02.3662	

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FIRST TRVERSE
POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

						- CHECK COMPUTATION -					
			°	'	"				°	'	"
α	N. Base, USN	to B	322	47	17.11	α	3	to 2			
2d		&	+298	54	07.00	3d		&			
α	2	to 1	261	41	24.11	α	3	to 1	81	41	24.37
Δα			+	00	00.26	Δα			-	00	00.26
			180	00	00.0				180	00	00.0
α	1	to 2	81	41	24.37	α	1	to 3	261	41	24.11

FIRST ANGLE OF TRIANGLE

Computations		Values in seconds		Logarithms		Values in seconds	
2.7 - 38	2d term + 0.0000	9.3017546		9.5124992		9.3017546	
1.88926				8.5124992			
D 1.2851				7.16111			
2.5377	3d term + 0.0000						
	Δα + 00.1889						

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POSITION COMPUTATION, ~~THIRD-ORDER~~ ^{FIRST} ~~TRIANGULATION~~ ^{TRAVERSE}

			CHECK COMPUTATION		
α	Δ GRAFLEX	117 01	35.68	β	to 2
		274 41	54.9		&
β		31 43	30.58	γ	75 21.78
					+ 00 00.60

α	Δ GRAFLEX	162 19	14.568	β	to 2
					&
β				γ	75 21.78
					+ 00 00.60

Left Column		Middle Column		Right Column	
2.250 811	B	8.512 411		2.250 811	B
sin α 9.720 8580		b 0.673 0309		sin α 9.720 8580	
N 8.509 6669		s ² 4.461 64		N 8.509 6669	
sec α 6.008 9998		sin ² α 9.441 71		sec α 6.008 9998	
C 0.720 00	-Δλ	0.470 3424 +02.9535	C	0.719 97	-Δλ
7.620 36 2d term +0.0000	sin 3(φ+φ')	9.304 2786	4.623 32	2d term +0.0000	sin 3(φ+φ')
h 1.3461	-Δα	9.774 6208 +00.595	h ³ 1.3461		-Δα
D 1.9876			D 1.9876		
3.3337 3d term +0.0000			3.3337	3d term +0.0000	
Δφ +04.7101				-Δφ -04.7101	

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FIRST TRAVEL
POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

SECRET

Check Computation: 0 - 1 - "

2	GRAFLEX to N. Base, U.S.N.	335	23	28.37	α	3	to 2			
2	&	+263	34	11.8-	3d α		&			
2	to 1	238	57	40.17	α	3	to 1	58	57	40.31
		+	00	00.14	Δα			-	00	00.14
		180	00	00.0				180	00	00.0
1	to 2	58	57	40.31	α	1	to 3	238	57	40.17

FIRST ANGLE OF TRIANGLE

11.37	31.564	2	GRAFLEX	162	19	14.568
11.37	31.784	1	Aoman Photo Topo	162	19	15.277

Logarithms	Values in seconds	Logarithms	Values in seconds	Logarithms	Values in seconds	Logarithms	Values in seconds
1.398 917		1.398 917		1.398 917		1.398 917	
1.3512 4910		1.3512 4910		1.3512 4910		1.3512 4910	
1.842 127		1.842 127		1.842 127		1.842 127	
2.197 87		2.197 87		2.197 87		2.197 87	
Sin α 9.865 78		Sin α 9.865 78		Sin α 9.865 78		Sin α 9.865 78	
C 0.720 00		-Δα 4.830 4691 -00.7087		C 0.720 00		-Δα 4.830 4691 +00.7087	
3.383 60	2d term 0.0000	Sin β 9.304 3047		3.383 60	2d term 0.0000	Sin β 9.304 3047	
h ² 9.247 25		-Δα 9.154 7738 -00.143		h ² 9.247 25		-Δα 9.154 7738 +00.143	
D 1.9876				D 1.9876			
1.2351	3d term +0.0000			1.2351	3d term +		
	Δδ -00.4205				-Δδ +00.4205		

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POSITION COMPUTATION, ^{First} ~~Third~~-ORDER TRIANGULATION

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		CHECK			COMPUTATION		
α	Δ A ELGIN to Δ GRAFLEX	277	00	43 52	α	3	to 2
	&	-196	38	34.52	β		&
β	to 1	133	39	17.84	α	3	to 1
			00	05.98	Δα		
		180	00	00.00			180 00 00.00

α	to Δ ELGIN	142	14	55.132	α	3	to 2
β	to Δ GRAFLEX	133	39	17.84	β		&
γ	to Δ ELGIN	142	14	55.132	γ		to 1
δ	to Δ GRAFLEX	133	39	17.84	δ		&

Sine		Cosine		Tangent	
α	9.718 89	0.009 0705	1.470 5900	6.184 82	0.184 82
β	6.625 10	0.776 3669	0.776 3669	2.887 9	2.887 9
γ	9.718 89	0.009 0705	1.470 5900	6.184 82	0.184 82
δ	6.625 10	0.776 3669	0.776 3669	2.887 9	2.887 9

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U. S. COAST AND GEODETIC SURVEY
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FIRST TRAVELER
POSITION COMPUTATION; THIRD-ORDER TRIANGULATION

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				←	Check Computation				→
	°	'	"						
N. Base USN to B RUNIT	322	47	17.11 ^x	α	3	to 2			
&	+344	17	02.5 [✓]	β	L	&	-		
2 to 1	307	04	19.61 ^x	α	3	to 1	127	04	21.64
+	00	00	02.03 [✓]	Δα	* Corrected & from B.L. Loop Comp.				
	180	00	00.0				180	00	00.0
1 to 2	127	04	21.64 [✓]	α	1	to 3	307	04	19.61

33 15 131 N. Base USN (104)	162	21	20.02	α	3	23.265	N. Base USN	162	21	09.99
-----------------------------	-----	----	-------	---	---	--------	-------------	-----	----	-------

Logarithm	Value in seconds
2.587 3323	20.00
9.180 1930	24.50
8.512 4993	
0.82 2476	
0.008 8914	
1.004 8275	
9.307 7223	
0.306 5477	
1.9851	
3.7391	
107.5339	

Logarithms	Value in seconds
2.587 3323	20.00
9.180 1930	24.50
8.512 4993	
0.82 2476	
0.008 8914	
1.004 8275	
9.307 7223	
0.306 5477	

Logarithms	Value in seconds
2.587 3323	20.00
9.180 1930	24.50
8.512 4993	
0.82 2476	
0.008 8914	
1.004 8275	
9.307 7223	
0.306 5477	

Logarithms	Value in seconds
2.587 3323	20.00
9.180 1930	24.50
8.512 4993	
0.82 2476	
0.008 8914	
1.004 8275	
9.307 7223	
0.306 5477	

* Not a permanent station

V.R.S.

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POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

SECRET

		"				check		"	
2	5.9	21	04	21	07	3	to 2		
		196	03	38.5	*		&		
		223	08	00.14	*	3	to 1	143	08
			00	04.76					04.76
								180	00
		143	08	04.76					

22	20.002	25	21	20.002	0	32	44	48	0	22	21	20.002
	00	31	24	23.759								23.759

27	162	21	43	161	0	38	15	73	0	1	4(A)	162	21	20.002
----	-----	----	----	-----	---	----	----	----	---	---	------	-----	----	--------

		LOGARITHMS		VALUES IN	
2	9.718182	2	9.718182	2	9.718182
3	8.507667	3	8.507667	3	8.507667
4	1.3758264	4	1.3758264	4	1.3758264
5	9.3015151	5	9.3015151	5	9.3015151
6	0.6773415	6	0.6773415	6	0.6773415
7	0.71695	7	0.71695	7	0.71695
8	6.43149	8	6.43149	8	6.43149
9	2.9846	9	2.9846	9	2.9846
10	1.9848	10	1.9848	10	1.9848
11	4.9744	11	4.9744	11	4.9744
12	31.2446	12	31.2446	12	31.2446

** This is a permanent station
Marked with a standard U.S.C+GS Disk
Stamped "TRANSVERSE STA. RUNIT"

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
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FIRST TRVERSE
POSITION COMPUTATION, ~~THIRD~~-ORDER TRIANGULATION

SECRET

	1	2	3	4	5	CHECK	COMPUTATION	→	6	7	8
1	A	GRAPEX	211	43	29.98	α	3	to 2			
2			+94	34	08.0-	β		&			
3		to 1	300		37.45	"	3	to 1	126	17	41 20
				00	03.27	Δα			-	00	03.22
			100	00	00 0				180	00	00.0

17

Logarithms	Values in seconds	Logarithms	Values in seconds
log 211.43 = 2.323 698		log 300 = 2.477 121	
log 94 = 1.973 127		log 100 = 2.000 000	
log 3.00 = 0.477 121			
log 0.94 = 0.973 127			
log 0.03 = 0.004 321			
log 0.08 = 0.103 174			
log 0.008947 = 0.952 251			
log 1.2031552 = 0.082 241			
log 304.1951 = 2.483 384			
log 0.5073503 = 0.705 245			
log 1.9876 = 0.297 655			
log 1.35 = 0.126 915			
log 11.5543 = 1.063 133			
3d term +		3d term +	
Δφ		Δφ	

574

* This point is located with stake Ameron 12 at the Gamma line thru tape traverse.

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U. S. COAST AND GEODETIC SURVEY
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TRVERSE
POSITION COMPUTATION, THIRD-ORDER TRIANGULATION

CHECK COMPUTATION

Station	Angle	Side	Angle	Side	Station	Angle	Side	Angle	Side
27	103	45.27	126	17	41.20	3	to 2		
			180	00	00	39	&		
2	101	71.40	126	17	45.27	3	to 1		
			4	00	04	07			
			180	00	00				
27	103	45.27	162	19	47.79	37	20.667	162	19
28	103	45.27	162	19	47.79	37	20.667	162	19
29	103	45.27	162	19	47.79	37	20.667	162	19
30	103	45.27	162	19	47.79	37	20.667	162	19
31	103	45.27	162	19	47.79	37	20.667	162	19
32	103	45.27	162	19	47.79	37	20.667	162	19
33	103	45.27	162	19	47.79	37	20.667	162	19
34	103	45.27	162	19	47.79	37	20.667	162	19
35	103	45.27	162	19	47.79	37	20.667	162	19
36	103	45.27	162	19	47.79	37	20.667	162	19
37	103	45.27	162	19	47.79	37	20.667	162	19
38	103	45.27	162	19	47.79	37	20.667	162	19
39	103	45.27	162	19	47.79	37	20.667	162	19
40	103	45.27	162	19	47.79	37	20.667	162	19
41	103	45.27	162	19	47.79	37	20.667	162	19
42	103	45.27	162	19	47.79	37	20.667	162	19
43	103	45.27	162	19	47.79	37	20.667	162	19
44	103	45.27	162	19	47.79	37	20.667	162	19
45	103	45.27	162	19	47.79	37	20.667	162	19
46	103	45.27	162	19	47.79	37	20.667	162	19
47	103	45.27	162	19	47.79	37	20.667	162	19
48	103	45.27	162	19	47.79	37	20.667	162	19
49	103	45.27	162	19	47.79	37	20.667	162	19
50	103	45.27	162	19	47.79	37	20.667	162	19
51	103	45.27	162	19	47.79	37	20.667	162	19
52	103	45.27	162	19	47.79	37	20.667	162	19
53	103	45.27	162	19	47.79	37	20.667	162	19
54	103	45.27	162	19	47.79	37	20.667	162	19
55	103	45.27	162	19	47.79	37	20.667	162	19
56	103	45.27	162	19	47.79	37	20.667	162	19
57	103	45.27	162	19	47.79	37	20.667	162	19
58	103	45.27	162	19	47.79	37	20.667	162	19
59	103	45.27	162	19	47.79	37	20.667	162	19
60	103	45.27	162	19	47.79	37	20.667	162	19
61	103	45.27	162	19	47.79	37	20.667	162	19
62	103	45.27	162	19	47.79	37	20.667	162	19
63	103	45.27	162	19	47.79	37	20.667	162	19
64	103	45.27	162	19	47.79	37	20.667	162	19
65	103	45.27	162	19	47.79	37	20.667	162	19
66	103	45.27	162	19	47.79	37	20.667	162	19
67	103	45.27	162	19	47.79	37	20.667	162	19
68	103	45.27	162	19	47.79	37	20.667	162	19
69	103	45.27	162	19	47.79	37	20.667	162	19
70	103	45.27	162	19	47.79	37	20.667	162	19
71	103	45.27	162	19	47.79	37	20.667	162	19
72	103	45.27	162	19	47.79	37	20.667	162	19
73	103	45.27	162	19	47.79	37	20.667	162	19
74	103	45.27	162	19	47.79	37	20.667	162	19
75	103	45.27	162	19	47.79	37	20.667	162	19
76	103	45.27	162	19	47.79	37	20.667	162	19
77	103	45.27	162	19	47.79	37	20.667	162	19
78	103	45.27	162	19	47.79	37	20.667	162	19
79	103	45.27	162	19	47.79	37	20.667	162	19
80	103	45.27	162	19	47.79	37	20.667	162	19
81	103	45.27	162	19	47.79	37	20.667	162	19
82	103	45.27	162	19	47.79	37	20.667	162	19
83	103	45.27	162	19	47.79	37	20.667	162	19
84	103	45.27	162	19	47.79	37	20.667	162	19
85	103	45.27	162	19	47.79	37	20.667	162	19
86	103	45.27	162	19	47.79	37	20.667	162	19
87	103	45.27	162	19	47.79	37	20.667	162	19
88	103	45.27	162	19	47.79	37	20.667	162	19
89	103	45.27	162	19	47.79	37	20.667	162	19
90	103	45.27	162	19	47.79	37	20.667	162	19
91	103	45.27	162	19	47.79	37	20.667	162	19
92	103	45.27	162	19	47.79	37	20.667	162	19
93	103	45.27	162	19	47.79	37	20.667	162	19
94	103	45.27	162	19	47.79	37	20.667	162	19
95	103	45.27	162	19	47.79	37	20.667	162	19
96	103	45.27	162	19	47.79	37	20.667	162	19
97	103	45.27	162	19	47.79	37	20.667	162	19
98	103	45.27	162	19	47.79	37	20.667	162	19
99	103	45.27	162	19	47.79	37	20.667	162	19
100	103	45.27	162	19	47.79	37	20.667	162	19

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This part is identical with
State Bijiiri II of the
Gamma Line, Invar Traverse

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DEPARTMENT OF COMMERCE
 U. S. COAST AND GEODETIC SURVEY
 Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

"CHECK ON INVERSE COMPUTATION"

α 2 to 3				α 3 to 2			
247				3 ^d L			
&				&			
2 to 1				α 3 to 1			
331 58 18.62				Δα			
+ 00 32.56				180 00 00.00			
180 00 00.00				180 00 00.00			
1 to 2				α 1 to 3			
101 58 01.00							
Clear Angle of Triangle							
202 1 6.74							
202 43.00							
202 54.75							
Logarithms				Logarithms			
9.022 0406				9.022 0406			
Sum				Sum			
2.765 757				2.765 757			
Log α				Log α			
9.745 237				9.745 237			
Sum				Sum			
4.85 237				4.85 237			
Log sin α				Log sin α			
9.745 237				9.745 237			
Sum				Sum			
2.765 757				2.765 757			
Log cos α				Log cos α			
9.745 237				9.745 237			
Sum				Sum			
2.765 757				2.765 757			
Log sec α				Log sec α			
9.745 237				9.745 237			
Sum				Sum			
2.765 757				2.765 757			
Log sin ² α				Log sin ² α			
9.745 237				9.745 237			
Sum				Sum			
2.765 757				2.765 757			
Log cos ² α				Log cos ² α			
9.745 237				9.745 237			
Sum				Sum			
2.765 757				2.765 757			
Arc-sin corr.				Arc-sin corr.			
(2)=K				(2)=K			
Δλ				Δλ			
7.500 705				7.500 705			
Δφ				Δφ			
5.912				5.912			
sin $\frac{1}{2}(\phi + \phi')$				sin $\frac{1}{2}(\phi + \phi')$			
7.500 705				7.500 705			
sec $\frac{\Delta\phi}{2}$				sec $\frac{\Delta\phi}{2}$			
(3)				(3)			
sec ² φ				sec ² φ			
(approx.)				(approx.)			
Δα				Δα			
-32.558				-32.558			
do				do			
(8)				(8)			
Arc-sin corr.				Arc-sin corr.			
for s				for s			
-				-			
(Δλ) ²				(Δλ) ²			
-				-			
for Δλ				for Δλ			
+				+			
F				F			
Total				Total			
(8)				(8)			
-Δλ				-Δλ			
-163.111				-163.111			

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SOLUTION COMPUTATION

in which $\log \Delta s = \log (s_1 - s_2)$; $\log \phi = \log \phi_1$; $\log \phi_2$ correction for arc to sin*; and $\log s = \log s_1 +$ correction for arc to sin*.

NAME		LOCATION	
1.	ϕ	$27^{\circ} 28' 23.11''$	$162^{\circ} 23' 59.152'' E$
2.	ϕ	$27^{\circ} 28' 20.987''$	$162^{\circ} 21' 16.041'' E$
$\Delta \phi$	$\phi_1 - \phi_2$	$2.124''$	$+ 02 \quad 43.111$
$\frac{\Delta \phi}{2}$		$1.062''$	$+ 01 \quad 21.556$
$\phi_m \left(\phi_1 + \frac{\Delta \phi}{2} \right)$		$27^{\circ} 28' 24.81''$	
$\Delta \phi$ (secs)		2.124	$+ 163.111$
log Ac		$2.212 \quad 4833 P$	$2.212 \quad 4833 P$
cor. arc to sin			0
log $\Delta \phi$		$2.212 \quad 4833$	$2.212 \quad 4833$
log cos $\frac{\Delta \phi}{2}$		$9.991 \quad 1714$	$9.991 \quad 1714$
colog B_m		$1.490 \quad 3320$	colog A
log s₁ cos $\left(\alpha + \frac{\alpha}{2} \right)$		$3.493 \quad 9867 P$	log s₁ sin $\left(\frac{\Delta \alpha}{2} \right)$
		$3.967 \quad 8802 P$	log s₁ cos $\left(\frac{\Delta \alpha}{2} \right)$
log $\Delta \alpha$		$9.022 \quad 0406$	log tan $\left(\frac{\Delta \alpha}{2} \right)$
log sin ϕ_m		$1.51 \quad 58 \quad 34.90$	$1.51 \quad 58 \quad 34.90$
log sin $\frac{\Delta \phi}{2}$		$9.671 \quad 9461$	log sin $\left(\frac{\Delta \alpha}{2} \right)$
log a		$9.945 \quad 8396$	log cos $\left(\frac{\Delta \alpha}{2} \right)$
a		$4.022 \quad 0406$	log s₂
b		0	0
$\Delta \alpha$ (secs.)		$4.022 \quad 0406$	$4.022 \quad 0406$
$\frac{\Delta \alpha}{2}$		$2.011 \quad 0203$	
$\alpha + \frac{\Delta \alpha}{2}$		$161^{\circ} 28' 34.90''$	
α (1 to 2)		$161^{\circ} 28' 34.90''$	
$\Delta \alpha$		$0.000 \quad 0000$	
α' (2 to 1)		$161^{\circ} 28' 34.90''$	

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Note. For log ϕ or $\log \phi_2$ for $\Delta \phi$ or $\Delta \alpha$ of 180° or 0°, omit all terms below the heavy line except those printed (in whole or in part) below. Use the correction tables on the back of this form for correction of $\log \phi$.

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Handwritten notes and scribbles at the top of the page.

in which $\log s_1 = \log (s_1 \cos \frac{\Delta\alpha}{2}) + \log s_2 \cos \frac{\Delta\alpha}{2}$; $\log s_2 = \log (s_2 \cos \frac{\Delta\alpha}{2}) + \log s_3 \sin^2 \frac{\Delta\alpha}{2}$; and $\log s = \log s_1 +$
 correction for arc length.

1. ϕ	11 23 41.1	200	262	21	1.202
2. ϕ'	11 23 41.1	200	262	21	1.202
$\Delta\phi$	0.000				59.555
$\frac{\Delta\phi}{2}$	0.000				59.794
ϕ_m ($\frac{\phi + \phi'}{2}$)	11 23 41.1				
$\Delta\phi$ (secs.)					119.588
log $\Delta\phi$	2.334				2.677 6876
cor. arc \sin					0
log $\Delta\phi_1$	2.334				2.077 6876
log $\cos \frac{\Delta\lambda}{2}$					9.991 0528
colog B_m	1.417				1.490 3328
log $s_1 \cos(\alpha \pm \frac{\Delta\alpha}{2})$	3.873				3.554 0732 p.
					3.873 8225 d.
log $\Delta\lambda$					9.680 2507
log $\sin \phi_m$					1.54 09 0736
log $\sec \frac{\Delta\phi}{2}$					9.624 4710
log a					9.954 2204
a					3.417 6024
b					c
$-\Delta\alpha$ (secs.)					3.417 6024
$-\frac{\Delta\alpha}{2}$					
$\frac{\Delta\alpha}{2}$					
α (1 to 2)					
$\Delta\alpha$					
α' (2 to 1)					

Note: For log supplied on a 11" paper, use the values below the heavy line except those printed in whole or in part in heavy type or those underscored, which are logarithms of tabulated angles.

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 28—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

" CHECK ON INVERSE COMPUTATION "

α	2	to 3				α	3	to 2			
$2^d \angle$		&	+			$3^d \angle$		&			
α	2	to 1	334	08	55.35	α	3	to 1			
$\Delta \alpha$			+	00	24.03	$\Delta \alpha$					
			180	00	00.00				180	00	00.00
α'	1	to 2	54	09	19.38	α'	1	to 3			
Three Angles of Triangle											
ϕ	11	37	26.8	11	614	ϕ	11	37	26.8	11	614
$\Delta \phi$					59.588	$\Delta \phi$					59.588
ϕ'	11	38	26.8	11	202	ϕ'	11	38	26.8	11	202
Logarithms			Logs			Logarithms			Logs		
α	3.9796022		1	0.000	11.352515	α	3.9796022		1	0.000	11.352515
$\cos \alpha$	9.9999998					$\cos \alpha$	9.9999998				
B	8.212477	Sum				B	8.212477	Sum			
(1)-h	2.3863012					(1)-h	2.3863012				
$\sin^2 \alpha$	9.27725	(5)				$\sin^2 \alpha$	9.27725	(5)			
C	0.71996	(6)	+		2.0776811	C	0.71996	(6)	+		2.0776811
(2)=K	7.83821	(7)	+		2.0776811	(2)=K	7.83821	(7)	+		2.0776811
$(\delta \phi)^2$	4.7726	$-\Delta \phi$			2.0776877	$(\delta \phi)^2$	4.7726	$-\Delta \phi$			2.0776877
D	1.9876	$\frac{A^2 \text{arc}^2 1''}{3}$			9.3024133	D	1.9876	$\frac{A^2 \text{arc}^2 1''}{3}$			9.3024133
(3)	6.7602	$\sec^2 \phi$			0	(3)	6.7602	$\sec^2 \phi$			0
-h	2.3863	(7)			1.3806944	-h	2.3863	(7)			1.3806944
$s^2 \sin^2 \alpha$	7.1182				"	$s^2 \sin^2 \alpha$	7.1182				"
E	5.6642	Arc-sin corr.			(8)	E	5.6642	Arc-sin corr.			(8)
(4)	5.2687	for s	-		$-\Delta \alpha$	(4)	5.2687	for s	-		$-\Delta \alpha$
		for $\Delta \lambda$	+		"			for $\Delta \lambda$	+		"
Total		(8)			$-\Delta \alpha$	Total		(8)			$-\Delta \alpha$

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NAVIGATION COMPUTATION

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in which $\log \Delta\lambda_1 = \log (\lambda' - \lambda)$ (corrected for arc to sin*), $\log s = \log s_1 +$ correction for arc to sin*.

		NAME OF STATION		
1.	ϕ	162	19	15.277 ✓
2.	ϕ'	162	14	25.580 ✓
$\Delta\phi (= \phi' - \phi)$		+ 04 49.697 ✓		
$\frac{\Delta\phi}{2}$		+ 02 24.848 ✓		
$\phi_m (= \phi + \frac{\Delta\phi}{2})$		1 31 28.420 ✓		
$\Delta\phi$ (secs.)		+ 289.697 ✓		
log $\Delta\phi$		2.178 440 ✓		
cor. arc--sin		0		
log $\Delta\phi_1$		2.178 440 ✓		
log $\cos \frac{\Delta\lambda}{2}$		9.999 9636 ✓		
colog B_m		1.487 5036 ✓		
log $(s_1 \cos(\alpha + \frac{\Delta\alpha}{2}))$		3.685 6050 ✓ (opposite in the table)		
log $\Delta\lambda$		2.461 9440 ✓		
log $\sin \phi_m$		9.205 135 143 ✓		
log $\sec \frac{\Delta\phi}{2}$		1.18 55 20.72 ✓		
log a		2.942 1447 ✓		
a		17.67 0963 ✓		
b		7 001 0963 ✓		
$-\Delta\alpha$ (secs.)		- 52.77 ✓		
$-\frac{\Delta\alpha}{2}$		- 26.385 ✓		
$\alpha + \frac{\Delta\alpha}{2}$		118 23 20.72 ✓		
α (1 to 2)		118 43 48.14 ✓		
$\Delta\alpha$		- 20.42 ✓		
180				
α' (2 to 1)		298 24 51.47 ✓		

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NOTE.—For log s up to 4.0 and for $\Delta\phi$ or $\Delta\lambda$ (or both) up to 30 (omit all terms below the heavy line except those printed (in whole or in part) in heavy type or those underscored, if using logarithms to 7 decimal places.

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

CHECK ON INVERSE COMPUTATION

α	2	to 3				α	3	to 2			
$2^d L$		&	+			$3^d L$		&	-		
α	2	to 1	298	54	51.47	α	3	to 1			
$\Delta\alpha$			+	00	58.49	$\Delta\alpha$					
			180	00	00.00				180	00	00.00
			18	55	49.96	α'	1	to 3			

First Angle 107.0000

ϕ	162	4	25.580	+		λ		
$\Delta\phi$	1	04	49.697	$\Delta\phi$		$\Delta\lambda$		
ϕ'	162	19	45.277	ϕ'		λ'		
Logarithms		Logs	0.699	Logarithms	(1)	Logs	9.699	Logarithms
$\sin^2 \alpha$	0.89436	$\cos^2 \alpha$	0.477	$\sec \phi$	6.00825	$\sin^2 \alpha'$		$\sec \phi'$
(2)=K	8.60825	(6)		Sum	2.4619439	(2)=K		Sum
$(\Delta\phi)^2$	7.0960	$-\Delta\phi + 157.7778$	(olog) E	$-\Delta\lambda$	2.4619439	$(\Delta\phi)^2$	$-\Delta\phi$	(olog) E
D	1.9941	$\frac{A^2 \sec^2 1''}{3}$	5.912	$\sin \frac{1}{2}(\phi + \phi')$	9.3051135	D	$\frac{\Delta\phi}{2}$	$\frac{A^2 \sec^2 1''}{3}$
(3)	6.3851	$\sec^2 \phi$		$\sec \frac{\Delta\phi}{2}$	0	(3)	$\sec^2 \phi$	$\sec \frac{\Delta\phi}{2}$
-h	2.1980	(7)		(approx.)	1.7610574	-h	(7)	(approx.)
$s^2 \sin^2 \alpha$	7.8866	do		$-\Delta\alpha$	-58.487	$s^2 \sin^2 \alpha'$	do	do
E	5.6646	Arc-sin corr.		(8)		E	Arc-sin corr.	(8)
(4)	5.7492	for s	-1.8	$(\Delta\lambda)^2$		(4)	for s	$(\Delta\lambda)^2$
		for $\Delta\lambda$	+1.4	F			for $\Delta\lambda$	F
Total	-0.4	(8)		$-\Delta\lambda$	-289.6969	Total	(8)	$\Delta\lambda$

INVERSE POSITION COMPUTATION



in which $\log \Delta s = \log (s_1 - s_2) = \log s_1 - \log s_2$ for arcs to \sin^* , and $\log s = \log s_1 +$ correction for arc to \sin^*

		NAME OF OBSERVER	
1.	ϕ	11 22 58.092	REEF POINT LIGHT
2.	ϕ'	11 39 21.939	ANSON'S LIGHT
$\Delta\phi$	$(\phi' - \phi)$	16 53.847	
$\frac{\Delta\phi}{2}$		8 26.924	
ϕ_m	$(\phi + \frac{\Delta\phi}{2})$	11 31 15.017	
$\Delta\phi$	(secs.)	16 53.847	
$\log \Delta\phi$		1.358 2717	$\log s$
cor. arc - \sin			2 582 0520 η
$\log \Delta\phi_1$		1.358 2717	
$\log \cos \frac{\Delta\lambda}{2}$		9.999 5768	2.582 0520
$\text{colog } B_w$		1.487 2442	$\log \cos$
$\log \left\{ s_1 \cos \left(\alpha - \frac{\Delta\alpha}{2} \right) \right\}$	(opposite to $\Delta\lambda$)	2.876 2727 η	2.991 1113
$\log \Delta\lambda$		2.582 2727	$\text{colog } A$
$\log \sin \phi_m$		9.301 2727	1.490 3324
$\log \sec \frac{\Delta\phi}{2}$		2.576 2727	$\log \left\{ s_1 \sin \left(\frac{\Delta\alpha}{2} \right) \right\}$
$\log a$		1.882 2727	4.063 4957 η
a		76.31	$\log \left\{ s_1 \cos \left(\frac{\Delta\alpha}{2} \right) \right\}$
b		10.00	2.846 2727 η
$-\Delta\alpha$ (secs.)		10.00	$\log \Delta\alpha$
$-\frac{\Delta\alpha}{2}$		5.00	1.217 2230
$a + \frac{\Delta\alpha}{2}$		266 81 46.87	$\log \sin$
α (1 to 2)		266 81 05.60	9.999 2029
$\Delta\alpha$		10.00	$\log \cos$
	180		2.781 9798
α' (2 to 1)		86 52 24.14	\log
			2.064 2428
			1 0
			9.064 2918

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NOTE: For logs up to 10 w. 10 Z. 6. 7. (6 1-4) 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200, 201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215, 216, 217, 218, 219, 220, 221, 222, 223, 224, 225, 226, 227, 228, 229, 230, 231, 232, 233, 234, 235, 236, 237, 238, 239, 240, 241, 242, 243, 244, 245, 246, 247, 248, 249, 250, 251, 252, 253, 254, 255, 256, 257, 258, 259, 260, 261, 262, 263, 264, 265, 266, 267, 268, 269, 270, 271, 272, 273, 274, 275, 276, 277, 278, 279, 280, 281, 282, 283, 284, 285, 286, 287, 288, 289, 290, 291, 292, 293, 294, 295, 296, 297, 298, 299, 300, 301, 302, 303, 304, 305, 306, 307, 308, 309, 310, 311, 312, 313, 314, 315, 316, 317, 318, 319, 320, 321, 322, 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737, 738, 739, 740, 741, 742, 743, 744, 745, 746, 747, 748, 749, 750, 751, 752, 753, 754, 755, 756, 757, 758, 759, 760, 761, 762, 763, 764, 765, 766, 767, 768, 769, 770, 771, 772, 773, 774, 775, 776, 777, 778, 779, 780, 781, 782, 783, 784, 785, 786, 787, 788, 789, 790, 791, 792, 793, 794, 795, 796, 797, 798, 799, 800, 801, 802, 803, 804, 805, 806, 807, 808, 809, 810, 811, 812, 813, 814, 815, 816, 817, 818, 819, 820, 821, 822, 823, 824, 825, 826, 827, 828, 829, 830, 831, 832, 833, 834, 835, 836, 837, 838, 839, 840, 841, 842, 843, 844, 845, 846, 847, 848, 849, 850, 851, 852, 853, 854, 855, 856, 857, 858, 859, 860, 861, 862, 863, 864, 865, 866, 867, 868, 869, 870, 871, 872, 873, 874, 875, 876, 877, 878, 879, 880, 881, 882, 883, 884, 885, 886, 887, 888, 889, 890, 891, 892, 893, 894, 895, 896, 897, 898, 899, 900, 901, 902, 903, 904, 905, 906, 907, 908, 909, 910, 911, 912, 913, 914, 915, 916, 917, 918, 919, 920, 921, 922, 923, 924, 925, 926, 927, 928, 929, 930, 931, 932, 933, 934, 935, 936, 937, 938, 939, 940, 941, 942, 943, 944, 945, 946, 947, 948, 949, 950, 951, 952, 953, 954, 955, 956, 957, 958, 959, 960, 961, 962, 963, 964, 965, 966, 967, 968, 969, 970, 971, 972, 973, 974, 975, 976, 977, 978, 979, 980, 981, 982, 983, 984, 985, 986, 987, 988, 989, 990, 991, 992, 993, 994, 995, 996, 997, 998, 999, 1000

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

CHECK ON INVERSE COMPUTATION

α	2	to 3				α	3	to 2			
$2^d L$		&	+			$3^d L$		&	-		
α	2	to 1	266	31	08.64	α	3	to 1			
$\Delta\alpha$			+	01	16.50	$\Delta\alpha$					
			180	00	00.00				180	00	00.00
α'	1	to 2	86	37	25.14	α'	1	to 3			

First angle of triangle

ϕ	11	32	58	095	2000	4th Tower	λ	162	14	54.051	ϕ	8	λ	
$\Delta\phi$	4	00	32	244			$\Delta\lambda$	+	06	21.710	$\Delta\phi$		$\Delta\lambda$	
ϕ'	11	35	20	934	1	1000	2nd Tower	λ'	162	21	16.741	ϕ'	1	λ'

Logarithms				Logs				Logarithms				Logs			
s	4.064	2928	(1)	$\frac{1}{2}(\phi+\phi')$	11.33	09.52	s	(1)	$\frac{1}{2}(\phi+\phi')$	9.699	$\frac{1}{2}(\phi+\phi')$	s	(1)	$\frac{1}{2}(\phi+\phi')$	
-1				Logarithms				cos α	(2)		Logarithms				
cos α					4.064	2928	B	Sum			K				
B	8		Sum				(-)		3		E			sin α	
h	1.360	0981	3				sin α	4.494	1780	(1)-h				A'	
s^2	8.28	59	(4)				A'	8.509	6675	(4)				(5)	
sin ² α	9.978	70	(5)		3	0.477	sec ϕ'	0.008	8936	(5)	-	3	0.477	sec ϕ'	
C	0.1710	(6)	+	cos ² α			Sum	2.582	0519	(6)	+		cos ² α	Sum	
(2)=K	8.844	49	(7)	+	(6)		Arc-sin		0	(7)	+	(6)		Arc-sin	
(6) ²	2.720	2	- $\Delta\phi$	22.440	(colog) E		- $\Delta\lambda$	2.582	0519	(6) ²	- $\Delta\phi$		(colog) E	$\Delta\lambda$	
D	1.9849	$\frac{\Delta\phi}{2}$		$\frac{A^2 \text{arc}^2 1''}{3}$	5.912	$\sin \frac{1}{2}(\phi+\phi')$	9.301	6120	D	$\frac{\Delta\phi}{2}$		$\frac{A^2 \text{arc}^2 1''}{3}$	5.912	$\sin \frac{1}{2}(\phi+\phi')$	
(3)	4.7051			sec ² ϕ		sec $\frac{\Delta\phi}{2}$		0	(3)			sec ² ϕ		sec $\frac{\Delta\phi}{2}$	
-h	1.3601			(7)		(approx.)	- $\Delta\alpha$	1.883	6639	-h			(7)	(approx.)	
$s^2 \sin^2 \alpha$	8.1270					do		"		$s^2 \sin^2 \alpha$			do	"	
E	5.6636	Arc-sin corr.				(8)		"		E	Arc-sin corr.			(8)	
(4)	5.1507	for s -	($\Delta\lambda$) ²			- $\Delta\alpha$	-76.500		(4)	for s -	($\Delta\lambda$) ²			- $\Delta\alpha$	
		for $\Delta\lambda$ +	F							for $\Delta\lambda$ +	F				
		Total	(8)			- $\Delta\lambda$	-381.9899		Total		(8)			$\Delta\lambda$	

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 682
Rev. Sept. 1942

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INSTRUCTIONS FOR COMPUTATION

$$s_1 \sin \left(\alpha + \frac{\Delta\alpha}{2} \right) = \frac{\Delta\lambda_1 \cos \phi_m}{\Delta\lambda_m}$$

$$s_1 \cos \left(\alpha + \frac{\Delta\alpha}{2} \right) = \frac{\Delta\phi_1 \cos \frac{\Delta\lambda}{2}}{B_m}$$

$$\alpha = \Delta\lambda \sin \phi_m \sec \frac{\Delta\phi}{2} + \text{H.A.S.}$$

DEPARTMENT OF COMMERCE

NOV 15 1942

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in which $\log \Delta\lambda_1 = \log (\lambda' - \lambda) - \text{correction for arc to sin}^*$; $\log \Delta\lambda_2 = \log (\lambda - \lambda') - \text{correction for arc to sin}^*$; and $\log s = \log s_1 + \text{correction for arc to sin}^*$.

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	NAME OF STATION							
1. ϕ	11	52	28.095	REEF PHEASANT LIAISON	λ	162	14	54.051 E
2. ϕ'	11	3	26.854	ACOMBY ZERO ZOWER	λ	162	19	11.614 E
$\Delta\phi (= \phi' - \phi)$								-04 17.563
$\frac{\Delta\phi}{2}$								-02 08.782
$\phi_m (= \phi + \frac{\Delta\phi}{2})$	11	33	27.47					
$\Delta\phi$ (secs.)			+208 789					-257.563
log $\Delta\phi$			2.429 362		log			2.410 8818 γ
cor. arc-sin								0
log $\Delta\phi_1$			2.429 362					2.410 8818
log $\cos \frac{\Delta\lambda}{2}$			9.999 999		log $\cos s_1$			9.991 0583
colog B_m			1.487 501		colog Δ			1.490 3327
log $\left\{ s_1 \cos \left(\alpha + \frac{\Delta\alpha}{2} \right) \right\}$			3.916 864	opposite in sign to $\Delta\alpha$	log $\left\{ s \sin \left(\alpha - \frac{\Delta\alpha}{2} \right) \right\}$			3.892 2728 γ
					log $\left\{ s \cos \left(\alpha + \frac{\Delta\alpha}{2} \right) \right\}$			3.916 8647 γ
log $\Delta\lambda$			2.410 8818	$\log \Delta\lambda = 1233$	log $\tan \left(\alpha + \frac{\Delta\alpha}{2} \right)$			9.975 4081
log $\sin \phi_m$			9.302 2765	$\log \sin \phi_m = 7577$	$\frac{\Delta\alpha}{2}$			223 22 43.27
log $\sec \frac{\Delta\phi}{2}$				$\log \sec \frac{\Delta\phi}{2} = 4810$	log $\sin \left(\alpha - \frac{\Delta\alpha}{2} \right)$			9.836 8411
log a			1.713 58		log $\cos \left(\alpha - \frac{\Delta\alpha}{2} \right)$			9.861 4330
a					log s			4.055 4317
b								0
$-\Delta\alpha$ (secs.)								4.055 4317
$-\frac{\Delta\alpha}{2}$								
$\alpha + \frac{\Delta\alpha}{2}$			223 22 43.27					
α (1 to 2)			223 22 43.27					
$\Delta\alpha$								
180								
α' (2 to 1)			43 23 09.14					

NOTE.—For logs up to 4.0 and for $\Delta\phi$ or $\Delta\lambda$ for both λ_1 or λ_2 limit all terms below the heavy line except those printed (in whole or in part) in heavy type or those underscored, if using logarithms to 7 decimal places.

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1941 20
FIELD NO. 10

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

"CHECK ON INVERSE COMPUTATION"

α	2	to 3				α	3	to 2			
$2^d L$		&	+			$3^d L$		&	-		
α	2	to 1	223	22	17.41	α	3	to 1			
$\Delta\alpha$			+	00	51.73	$\Delta\alpha$					
			180	00	00.00				180	00	00.00
α'	1	to 2	43	23	09.14	α'	1	to 3			

First Angle of Triangle

λ	162	14	54.051	ϕ	3	λ		
$\Delta\lambda$	+	04	17.562	$\Delta\phi$		$\Delta\lambda$		
λ'	162	19	11.613	ϕ'	1	λ'		

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Page 1

Logarithms				Logs	Logarithms				Logs			
s	4.055 4317	(1) -	268.7915	$\frac{1}{2}$	9.699	$\frac{1}{2}(\phi+\phi')$	11 35 12.47	s	(1)	$\frac{1}{2}$	9.699	$\frac{1}{2}(\phi+\phi')$
$\cos \alpha$	7.861 4844	(2) +	00317	s^2		Logarithms		$\cos \alpha$	(2) +	s^2	Logarithms	
B		Sum		K			4.055 4317	B	Sum	K	s	
(1)-h	2.427 4555	(3) +	00007	F		$\sin \alpha$	4.816 2810	(1)-h	(3) +	F		
s^2		(4) -	00001	(5)		A'	8.517 2469	s^2	(4)	(5)	A'	
$\sin^2 \alpha$	9.673 57	(5) -		3	0.477	$\sec \phi'$	0.008 9998	$\sin^2 \alpha$	(5) -	3	0.477	$\sec \phi'$
C	0.717 10	(6) +		$\cos^2 \alpha$		Sum	2.410 8819	C	(6) +	$\cos^2 \alpha$	Sum	
(2)=K	8.501 53	(7) +		(6)		Arc-sin corr.		(2)=K	(7) +	(6)	Arc-sin corr.	
$(\delta\phi)^2$	4.8588	$-\Delta\phi$	-268.7592	(colog) E		$\Delta\lambda$	2.410 8818'	$(\delta\phi)^2$	$-\Delta\phi$	(colog) E	$\Delta\lambda$	
D	1.9849	$\frac{\Delta\phi}{2}$		$\frac{A^2 \sec^2 1''}{3}$	5.912	$\sin \frac{1}{2}(\phi+\phi')$	9.302 8765	D	$\frac{\Delta\phi}{2}$	$\frac{A^2 \sec^2 1''}{3}$	5.912	$\sin \frac{1}{2}(\phi+\phi')$
(3)	6.8437			$\sec^2 \phi$		$\sec \frac{\Delta\phi}{2}$	1	(3)		$\sec^2 \phi$		
-h	2.4294			(7)		(approx.) $-\Delta\alpha$	1.713 7584	-h		(7)	(approx.) $-\Delta\alpha$	
$s^2 \sin^2 \alpha$	7.7844			do		do	51.73	$s^2 \sin^2 \alpha$		do		
E	5.6636	Arc-sin corr.		(8)				E	Arc-sin corr.	(8)		
(4)	5.8774	for s	-2.3	$(\Delta\lambda)^2$		$-\Delta\alpha$	-51.73	(4)	for s		$(\Delta\lambda)^2$	$-\Delta\alpha$
		for $\Delta\lambda$	+1.1	F					for $\Delta\lambda$	+	F	
		Total	-1.2	(8)		$-\Delta\lambda$	-257.5620		Total		(8)	$\Delta\lambda$

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 662
Rev. Sept 1942

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INVERSE POSITION COMPUTATION

$$s_1 \sin \left(\alpha + \frac{\Delta\alpha}{2} \right) = \frac{\Delta\lambda_1 \cos \phi_m}{\Delta\alpha}$$

$$s_1 \cos \left(\alpha + \frac{\Delta\alpha}{2} \right) = \frac{\Delta\phi_1 \cos \lambda_1}{\Delta\alpha}$$

$$\Delta\alpha = \Delta\lambda \sin \phi_m \sec \frac{\Delta\phi}{2} - F \Delta\lambda^2$$

in which $\log \Delta\lambda_1 = \log (\lambda' - \lambda) - \text{correction for arc to sin}^*$; $\log \Delta\phi_1 = \log (\phi' - \phi) - \text{correction for arc to sin}^*$; and $\log s = \log s_1 + \text{correction for arc to sin}^*$.

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		NAME OF STATION	
1.	ϕ	11 32 38.095	REEF PHOTO TOWER
2.	ϕ'	11 40 09.282	ENGEA ZERO TOWER
$\Delta\phi (= \phi' - \phi)$		+ 07 41.187	
$\frac{\Delta\phi}{2}$		+ 03 50.594	
$\phi_m (= \phi + \frac{\Delta\phi}{2})$		11 36 38.958	
$\Delta\phi$ (secs.)		+ 431.687	
$\log \Delta\phi$		2.635 1690	
cor. arc-sin		-	
$\log \Delta\phi_1$		2.635 1690	
$\log \cos \frac{\Delta\lambda}{2}$			
colog B_m		1.487 5025	
$\log \left\{ s_1 \cos \left(\alpha + \frac{\Delta\alpha}{2} \right) \right\}$		4.122 6715	(opposite in sign to $\Delta\phi$)
$\log \Delta\lambda$		1.454 4027	
$\log \sin \phi_m$		9.303 7124	
$\log \sec \frac{\Delta\phi}{2}$			
$\log a$		0.758 1517	
a		+ 05.3	
b			
$-\Delta\alpha$ (secs.)		+ 0.0	
$-\frac{\Delta\alpha}{2}$		+ 0.0	
$\alpha + \frac{\Delta\alpha}{2}$		76 11 46.47	
α (1 to 2)		76 11 49.12	
$\Delta\alpha$		2.65	
180			
α' (2 to 1)		356 6 43.40	

Refer to the back of this form for correction of

NOTE.—For log s up to 4.0 and for $\Delta\phi$ or $\Delta\lambda$ (or both) up to 3 omit all terms below the heavy line except those printed (in whole or in part) in heavy type or those underlined. Logarithms to 7 decimal places.

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

"CHECK ON" INVERSE COMPUTATION

α	2	to 3				α	3	to 2					
$2^d L$		&	+			$3^d L$		&	-				
α	2	to 1	176	16	49.13	α	3	to 1					
$\Delta\alpha$			-	00	05.73	$\Delta\alpha$							
			180	00	00.00				180	00	00.00		
α	1		336	16	43.40	α'	1						
First Angle of Triangle													
ϕ	11	32	58.090	2	155.6	Tower	X	162	14	54.051	ϕ		8
$\Delta\phi$			+ 07		28.87	$\Delta\lambda$				28.471	$\Delta\phi$		
ϕ'	11	40	09.782	1	155.6	Tower	X	162	14	25.580	ϕ'		1
Logarithms			4	22	99.00	Logs			9	699	Logarithms		
$\cos \alpha$						$\cos \alpha$							
$\sin^2 \alpha$						$\sin^2 \alpha$							
C						C							
$(2)=K$						$(2)=K$							
D						D							
(3)						(3)							
-h						-h							
$s^2 \sin^2 \alpha$						$s^2 \sin^2 \alpha$							
E						E							
(4)						(4)							
Total						Total							

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OFFICIAL USE ONLY GEOGRAPHIC POSITIONS

North American Triangulation Station *Station 7414141*

STATION	ELEVATION	EASTING	NORTHING	DISTANCE	DISTANCE	
					METERS	FEET

FREE U.S.N. 1204	250	170	50	315	385	600
161	288	200	85	370	382	502

162	285	200	82	370	382	502
163	285	200	82	370	382	502

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DEPARTMENT OF COMMERCE
 U. S. COAST AND GEODETIC SURVEY
 Form 25
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COMPUTATION OF TRIANGLES

(State)

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 Pacific Southwest Region

U. S. GOVERNMENT PRINTING OFFICE: 1940 O-308402		ADJUSTED Spherical Angles	SPHERICAL ANGLE	SPHERICAL RADIUS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3						3 824 7980 ✓
1	REEF LIGHT	104 01 20.00	0.03	0598	0 251 4848 ✓	
2	W. Base LIGHT	112 02 16.00	0.04	16 09	9 980 8554 ✓	
3	Land LIGHT	104 01 20.00	0.03	5793	4 799 1263 ✓	
1-3						4 057 1382 ✓
1-2						3 875 4091 ✓
			0.20	0000		
2-3						4 057 1382
1	LAND LIGHT	106 01 20.00	0.05	1356	0 140 9749 ✓	
2	REEF LIGHT	108 04 00.00	0.08	5 54	9 742 8155 ✓	
3	LAND LIGHT	106 01 20.00	0.04	1690	9 993 1739 ✓	
1-3						3 946 9286 ✓
1-2						4 191 2870 ✓
			0.25	0000		
2-3						4 191 2870
1	LAND LIGHT	108 04 00.00	0.23	8853	0 153 7422 ✓	
2	REEF LIGHT	108 04 00.00	0.24	2610	9 905 6860 ✓	
3	LAND LIGHT	108 04 00.00	0.24	6537	0 995 5719 ✓	
1-3						4 250 7152 ✓
1-2						4 340 6011 ✓
			0.70	0000		
2-3						4 250 7152
1	PINNACLES LIGHT	113 56 00.00	0.07	0617	0 539 0510 ✓	
2	LANTERNA LIGHT	111 50 00.00	0.08	0439	9 571 6653 ✓	
3	LAND LIGHT	111 50 00.00	0.07	0944	9 843 0745 ✓	
1-3						2 86 4315 ✓
1-2						7 132 8407 ✓
			0.23	0000		

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 25
Ed. Nov. 1946

COMPUTATION OF TRIANGLES

State

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Pacific Southwest Region

STATION	Adjusted	ANGLE	PER	SHEER	PLANE ANGLE	LOGARITHM
				EXCESS	AND DISTANCE	
2-3						4.250 7152
1 STEEL USV	10.24	10.24		0.02	50.43	0.000 1653 ✓
2 LANTANA USV	10.24	10.24		0.02	44.95	8.985 1647 ✓
3 LILAC USV	10.24	10.24		0.02	34.62	9.996 6319 ✓
1-3						3.236 0452 ✓
1-2						4.247 5124 ✓
		10.28		0.08	0.00	
2-3						
1						
2						
3						
1-3						
1-2						
2-3						
1						
2						
3						
1-3						
1-2						
2-3						
1						
2						
3						
1-3						
1-2						

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SECOND

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, ~~SECOND~~-ORDER TRIANGULATION

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α	2	to 8	240	38	43.70	α	8	to 2	60	40	24.82
$2^d L$		& •	+05	32	44.98	$3^d L$		&	-82	52	24.65
α	2	to 1	246	11	28.68	α	8	to 1	337	48	00.17
$-\Delta\alpha$			+	01	45.30	$\Delta\alpha$			+		04.25
			180	00	00.00				180	00	00.00
α'	1	to 2	66	13	13.98	α'	1	to 8			

~~SECRET~~

Final Angle of Triangle

ϕ			62	3	50.696	ϕ					
$\Delta\phi$				02	2.52	$\Delta\phi$					
ϕ'			62	22	44.57	ϕ'			62		

Logarithms		Logs	Logarithms		Logs
(1)	4.247 524	9.899	(1)	3.036 1452	9.900
(2)	9.247 524		(2)	6.337 060	
(3)	6.709 68		(3)	5.410 7	
(4)	6.445 8		(4)	3.004 4	
(5)	2.366 1		(5)	1.715 1	
(6)	9.247 524		(6)	6.337 060	
(7)	9.247 524		(7)	6.337 060	
(8)	5.751		(8)	1.571	

Sum	$\Delta\lambda$	$\Delta\alpha$	$\sin(\phi+\phi')$	$\sec \frac{\Delta\phi}{2}$	(approx.) $-\Delta\alpha$	do
2.727 2276	2.727 2276	2.022 4118	9.295 1842	5.912	2.022 4118	105.2960
1.571	1.571	1.571	9.296 6672	5.912	0.628 3627	4.2497

Sum	$\Delta\lambda$	$\Delta\alpha$	$\sin(\phi+\phi')$	$\sec \frac{\Delta\phi}{2}$	(approx.) $-\Delta\alpha$	do
2.727 2276	2.727 2276	2.022 4118	9.295 1842	5.912	2.022 4118	105.2960
1.571	1.571	1.571	9.296 6672	5.912	0.628 3627	4.2497

Sum	$\Delta\lambda$	$\Delta\alpha$	$\sin(\phi+\phi')$	$\sec \frac{\Delta\phi}{2}$	(approx.) $-\Delta\alpha$	do
2.727 2276	2.727 2276	2.022 4118	9.295 1842	5.912	2.022 4118	105.2960
1.571	1.571	1.571	9.296 6672	5.912	0.628 3627	4.2497

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SECOND POSITION COMPUTATION, FIRST-ORDER TRIANGULATION

DEPARTMENT OF COMMERCE U. S. COAST AND GEODETIC SURVEY FORM 26—Rev. Apr. 11, 1930

Table with columns for angles (alpha, 2dL, alpha, Delta alpha, alpha', 1) and their corresponding values (e.g., 2 to 3, 327 56 52.40, 147 57 15.78).

First Angle of Triangle

Table with columns for angles (alpha, alpha') and values (e.g., 254 50 20.67, 288 54 26.70).

Main calculation table with columns for Logarithms, Logarithms, Logarithms, Logarithms, and various trigonometric functions like sin, cos, sec, and Arc-sin corr.

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 26—Rev. Apr. 11, 1930

POSITION COMPUTATION, ^{SECOND} FIRST-ORDER TRIANGULATION

Left Triangle					Right Triangle										
α	2	to 3	288	54	26.69	α	3	to 2	108	55	37.82				
2^dL		&	+33	34	51.62	3^dL		&	100	07	54.99				
α	2	to 1	322	29	18.31	α	3	to 1	008	47	42.83				
$\Delta\alpha$				01	02.12	$\Delta\alpha$					8.75				
			180	00	00.00				180	00	00.00				
α	1	to 2	42	00	00.43	α	1	to 3	7	55	37.66				
Angles of Triangle						Angles of Triangle									
ϕ	22	58	42	00	00	ϕ	22	58	42	00	00				
$\Delta\lambda$			05	12.042		$\Delta\lambda$			04	40.734					
λ	162	22	22.847			λ	162	22	22.847						
Logarithms				Logs				Logarithms				Logs			
				(1) = h								(1) = h			
				A'				cos α				cos α			
				sec ϕ'				B				B			
				Sum				(1) = h				(1) = h			
				Arc-sin corr.				sin α				sin α			
				$\Delta\lambda$				sin α				sin α			
				sec $\frac{\Delta\phi}{2}$				sin α				sin α			
				(approx.)				sin α				sin α			
				do				sin α				sin α			
				(8)				sin α				sin α			
				Arc-sin corr.				sin α				sin α			
				for s				sin α				sin α			
				for $\Delta\lambda$				sin α				sin α			
				Total				sin α				sin α			

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α	2	to 3	322	29	18.31	α	3	to 2	142	30	20.43
$2^d \angle$		&	+53	35	26.33	$3^d \angle$		&	-81	49	55.61
α	2	to 1	16	04	44.64	α	3	to 1	60	40	24.82
$\Delta \alpha$					39.70	$\Delta \alpha$				01	41.12
			180	00	00.00				180	00	00.00
α'	1	to 2	196	04	04.94	α'	1	to 3	240	38	43.70
			44	34	38.76						
First Angle of Triangle						"					
ϕ	1	20	24	185		ϕ	1	20	24	185	
$\Delta \phi$						$\Delta \phi$					
ϕ'	11	20	24	185		ϕ'	11	20	24	185	

Logarithms		logs		Logarithms		logs	
$\log a$	4.340 6011	(1)	+ 6.851 255	$\log b$	4.340 6011	(1)	+ 6.851 255
$\log B$	8.512 4997	Sum	6.851 447	$\log B$	8.512 4997	Sum	6.851 447
$\log h$	2.835 1901	(3)	+ 6.664 2	$\log h$	2.835 1901	(3)	+ 6.664 2
$\log \sin^2 \alpha$	7.566 0	(6)		$\log \sin^2 \alpha$	7.566 0	(6)	
$\log E$	5.663 5	(4)		$\log E$	5.663 5	(4)	
$\log \Delta \lambda$	6.065 3	(4)		$\log \Delta \lambda$	6.065 3	(4)	
$\log \Delta \alpha$				$\log \Delta \alpha$			
Total	- 7.8	(8)		Total	- 1.0	(8)	

$\Delta \lambda$	200	10	79
$\Delta \alpha$	39	70	22

REF ID: A66585
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Sheet 3 of 3

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GEOGRAPHIC POSITIONS

Locality **KWIKTOK, Marshall Islands** 2nd order triangulation **Astro Datum** State **Pacific Ocean** **USS BOWDITCH - 1944** Archive No. **305697/1**

STATION	LATITUDE	LONGITUDE	ELEVATION Meters	AZIMUTH	DISTANCE Meters	ALTIMETER	STATION	DISTANCE Meters	ALTIMETER
PETUNIA	11 38 59.362	1823 057	162 09 40.138	1215.740	239 34 23.79	59 34 48.57	SAGE	4305.72	3-634 0460
					243 33 46.69	63 34 28.48	ZIRNIA	6990.82	3-844 5280
					270 33 20.46	90 34 32.30	ASTER	10,777.10	4-032 5004
PETUNIA	11 38 59.362	1823 057	162 09 40.138	1215.740	59 34 48.57	239 34 23.79	PETUNIA	4305.72	3-634 0459
					243 33 46.69	63 34 28.48	ZIRNIA	6990.82	3-844 5280
					270 33 20.46	90 34 32.30	ASTER	10,777.10	4-032 5004
					291 10 00.03	113 33 10.10	CAMKILIA	11,271.60	4-051 9852
					325 30 39.85	145 30 45.84	REEP	12,355.80	4-241 1290
ZIRNIA	11 38 59.362	1823 057	162 09 40.138	1215.740	63 34 28.48	243 33 46.69	PETUNIA	6990.82	3-844 5280
					69 34 58.90	249 34 41.89	SAGE	4712.13	3-433 3102
					97 18 12.07	307 38 11.17	ASTER	5545.29	3-743 9238
PETUNIA	11 38 59.362	1823 057	162 09 40.138	1215.740	63 34 28.48	243 33 46.69	PETUNIA	6990.82	3-844 5280
					69 34 58.90	249 34 41.89	SAGE	4712.13	3-433 3102
					97 18 12.07	307 38 11.17	ASTER	5545.29	3-743 9238
					291 10 00.03	113 33 10.10	CAMKILIA	11,271.60	4-051 9852
					325 30 39.85	145 30 45.84	REEP	12,355.80	4-241 1290
CAMKILIA	11 37 57.805	1776 116	162 17 29.721	900.279	03 09 27.99	183 09 24.19	REEP	10,445.70	4-017 6901
					111 11 10.10	291 10 00.03	SAGE	11,271.60	4-051 9852
					117 22 47.25	297 22 24.29	ASTER	3081.83	3-589 0368
					302 57 25.15	122 57 58.18	GARDENIA	5922.17	3-772 4806
					321 39 33.90	141 40 18.15	NORTH BAKE	10,753.90	4-031 5685
GARDENIA	11 36 12.938	397.533	162 20 13.749	416.526	37 41 02.45	217 40 25.76	REEP	9069.22	3-957 5698
					122 57 58.18	302 57 25.15	CAMKILIA	5922.17	3-772 4806
					341 55 44.00	161 55 55.27	NORTH BAKE	5483.79	3-739 0812

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FINAL VALUES GEOGRAPHIC POSITIONS

ENIWETOK, Marshall Islands 2nd order triangulation Astro Datum State Pacific Ocean USS BOWDITCH - 1944 Archive No. 305697/1

STATION	LATITUDE	LONGITUDE	AZIMUTH	BACK AZIMUTH	RELATIONS	STATION VALUE	STATION VALUE		
ENIWETOK ASTRA PIKE	11 33 29.480	172 01 160 23 10.250	310.575						
SOUTH BASE	11 33 21.264	172 01 09.890	299.667	334 44 37.63	58 44 57.70	ENIWETOK ASTRA PIKE	12.747	2.105	4216
				324 59 12.56	145 59 43.57	SOUTH BASE	2981.43	2.474	4222
				327 56 52.40	147 57 15.77	SAND	6680.87	3.824	8331
				74 50 08.53	254 49 20.67	REEF	7506.61	3.875	4441
				141 40 18.15	321 39 33.90	CAMELLIA	10,753.90	4.031	5685
				161 55 55.27	341 55 44.00	GARDENIA	5483.79	1.739	5812
				129 0 0.64	149 0 44.01	SAND	370.40	0.168	4048
				357 21 58.31	179 22 01.87	LILAC	11,870.00	4.077	1832
				91 4 50.06	273 1 43.10	REEF	7506.61	3.875	4441
				145 59 43.57	125 59 42.56	NORTH BASE	7981.43	2.474	4222
				145 0 45.84	125 0 39.85	SAGE	17,555.80	4.224	2200
				166 43 51.92	146 43 32.85	ASTER	12,519.30	4.097	5798
				183 09 24.19	03 09 27.99	CAMELLIA	10,415.70	4.017	6901
				217 40 25.76	37 41 02.45	GARDENIA	9069.22	3.957	5698
				254 49 20.67	74 50 08.53	NORTH BASE	7506.61	3.875	4441
				273 14 52.17	93 15 50.99	SOUTH BASE	8927.19	3.950	7152
				288 54 26.68	108 55 37.81	SAND	11,407.00	4.057	1732
				322 29 18.30	142 30 20.43	LILAC	15,535.40	4.191	3221
				16 04 44.63	196 04 04.93	LANTANA	21,909.70	4.340	6362

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FINAL VALUES

GEOGRAPHIC POSITIONS

Locality	Station	Latitude	Longitude	State	Distance	Coordinates
		Seconds in Meters	Seconds in Meters		Meters	
BARD	LILAC	11 30 18.966	162 23 06.883	208.596	8728.98	3.940 9636
	LANTANA			224.09	27.85	4.383 7598
	HICKY			108.51	29.68	3.057 1734
	SOUTH BASE			147.57	15.17	3.872 8331
	SOUTH BASE			149.31	12.01	3.702 8504
LILAC	PRIVILEGE			184.30	27.84	3.861 7665
	LANTANA			210.48	13.69	4.250 7502
	HICKY			122.31	28.30	3.197 1721
	SOUTH BASE			177.22	01.87	4.372 9854
	SAND			188.17	14.07	3.947 9636
SAND	STEEL			157.48	04.42	3.236 8804
	HICKY			122.31	28.30	3.197 1721
	LANTANA			210.48	13.69	4.250 7502
	PRIVILEGE			184.30	27.84	3.861 7665
	STEEL			157.48	04.42	3.236 8804
STEEL	PRIVILEGE			206.48	18.70	3.779 7427
	LANTANA			246.11	28.67	4.247 8474
	LILAC			157.48	04.42	3.236 8804
	LANTANA			262.32	38.16	4.132 8757
	LILAC			196.30	11.84	3.861 7665
PRIVILEGE	STEEL			206.48	18.70	3.779 7427
	LANTANA			262.32	38.16	4.132 8757

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 DESCRIPTION OF TRIGONOMETRIC LEVELING STATION

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NAME OF STATION: ELGIN STATE: Marshall Islands COUNTY: Eniwetok Atoll
 CHIEF OF PARTY: R. I. F. YEAR: 1948 LOCALITY: Engebi Island

Surface-station mark, <u>None</u> Underground-station mark, <u>None</u> Reference mark, <u>None</u> Reference mark, <u>None</u> Azimuth mark, <u>None</u> Witness mark, <u>None</u> Height of light above station mark, <u>None</u> feet Height of telescope above station mark, <u>2</u> meters	DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND												
	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 33%;">DISTANCE</th> <th style="width: 33%;">DIRECTION</th> <th style="width: 33%;">AZIMUTH</th> </tr> </thead> <tbody> <tr> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table>	DISTANCE	DIRECTION	AZIMUTH									
DISTANCE	DIRECTION	AZIMUTH											

Detailed description: The station is located on Engebi Island, approximately 700 feet north of the south end of the island, 100 feet east of the seaward side, 300 feet west of the lagoon side, and 45 feet south of the southeast corner of a small quonset hut. The disk is a standard USC&GS station disk set in a 12 by 12 inch concrete mark that projects 2 inches above the surface of the ground and is stamped ELGIN.

Described by G.R.S.

Marked by G.A.J.

NOTE.—The initial direction near the station scheme station

refers to pages 108 and 109, Special Publication No. 120, or to pages 112 and 113, Special Publication No. 145. ... 11-5761
 nearest meter only, when no trigonometric leveling is being done.

~~SECRET~~
 DESCRIPTION OF TRIGONOMETRIC LEVELING STATION

NAME OF STATION: GRAFLEA STATE: Marshall Islands COUNTY: Eniwetok Atoll
 CHIEF OF PARTY: R. C. P. YEAR: 1948 LOCALITY: Acomon Island

Surface-station mark, <u>None</u> Underground-station mark, <u>None</u> Reference mark, <u>None</u> Reference mark, <u>None</u> Azimuth mark, <u>None</u> Witness mark, <u>None</u> Height of light above station mark, <u>None</u> meters Height of telescope above station mark, <u>18</u> meters	DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND												
	<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 33%;">DISTANCE</th> <th style="width: 33%;">DIRECTION</th> <th style="width: 33%;">AZIMUTH</th> </tr> </thead> <tbody> <tr> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table>	DISTANCE	DIRECTION	AZIMUTH									
DISTANCE	DIRECTION	AZIMUTH											

Detailed description: The station is located on the north end of Acomon Island, about 500 feet west of the east shoreline of the island, 25 feet south of the north shoreline, and 75 feet west of the northwest leg of the photo-tower. The disk is a standard USC&GS station disk set in a 12 by 12 inch concrete mark that projects 6 inches above the surface of the ground and is stamped GRAFLEA.

No reference marks were established at this station.

Described by G.R.S.

Marked by G.A.J.

NOTE.—The initial direction near the station scheme station

refers to pages 108 and 109, Special Publication No. 120, or to pages 112 and 113, Special Publication No. 145. ... 11-5761
 nearest meter only, when no trigonometric leveling is being done.

M.S.

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~~SECRET~~
DESCRIPTION OF STATION

NAME OF STATION: CORAL
CHIEF OF PARTY: R. L. Pfau
Surface-station mark, Note,*
Underground-station mark, Note,*
Reference mark, Note,*
Reference mark, Note,*
Azimuth mark, Note,*
Witness mark, Note,*
Height of light above station mark meters
Height of telescope above station mark meters

STATE: Marshall Islands COUNTY: Eniwetok Atoll
YEAR: 1948 LOCALITY: Eniwetok Lagoon
DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND

OBJECT	DISTANCE	DIRECTION	AZIMUTH
This is a <u>reference</u> station.			

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Detailed description: The station is located atop a circular concrete cell that is 15 feet in diameter, about 2 miles east-southeast of the reef flat tower, about 5 miles west of Runit Island, and 0.15 mile west of bouy No. 15. The base is a standard USC&GS station disk set in the center of the structure, about 1 foot above the high water mark, stamped CORAL, and is surrounded by a sheet metal walkway projects 10 feet out of the water.

Described by G.R.S.

Marked by G.H.J.

NOTE.—The initial direction must be to main meridian.

* Refers to pages 108 and 109, Special Publication No. 120, or to pages 112 and 113, Special Publication No. 145.
† To nearest meter only when no trigonometric leveling is being done.

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DESCRIPTION OF STATION

NAME OF STATION: REEF PHOTO TOWER
CHIEF OF PARTY: R. L. Pfau
Surface-station mark, Note,*
Underground-station mark, Note,*
Reference mark, Note,*
Reference mark, Note,*
Azimuth mark, Note,*
Witness mark, Note,*
Height of light above station mark meters
Height of telescope above station mark meters

STATE: Marshall Islands COUNTY: Eniwetok Atoll
YEAR: 1948 LOCALITY: Eniwetok Lagoon
DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND

OBJECT	DISTANCE	DIRECTION	AZIMUTH
This is a <u>reference</u> station.			

Detailed description: The station is a 4 leg steel structure constructed atop 4 concrete ^{steel piles encased in} pilings located on a reef that is approximately 7 statute miles south of Engepi, 7 miles west of the north end of Runit Island, and 2 miles west northwest of station CORAL. Directions were obtained on a temporary point established at the intersection of the diagonals of the opposite legs of the tower and this point was marked on a wood deck that was constructed about 5 feet above the high water mark. No permanent mark was set and the station was not occupied due to the tide present.

Described by G.R.S.

Marked by

NOTE.—The initial direction must be to main meridian.

* Refers to pages 108 and 109, Special Publication No. 120, or to pages 112 and 113, Special Publication No. 145.
† To nearest meter only when no trigonometric leveling is being done.

DESCRIPTION OF STATION

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NAME OF STATION: KODAK STATE: Marshall Islands COUNTY: Eniwetok Atoll
 CHIEF OF PARTY: R. L. Pfeu YEAR: 1945 LOCALITY: Aniyaanii Island

DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND			
MARK	DISTANCE	DIRECTION	AZIMUTH
CC&I	0 meters	0° 00' 00.0"	
Reference mark No. 1	17.495	113 01 16	
Aniyaanii photo tower	21.425	202 56 19.6	
Reference mark No. 2	23.777	224 07 02	

Detailed description: The station is located about 100 feet south of the end of vegetation in a small clearing on the lagoon side of Aniyaanii Island, 80 feet south of the north edge of the clearing, 125 feet east of the high-water mark on the lagoon beach, and 75.5 feet north of the northwest leg of the photo tower. The disk is a standard USC&GS station disk set in a 12 by 12 inch concrete mark that is 1 inch above the surface of the ground and is stamped KODAK.

Reference mark No. 1 is 47.394 feet east of the station, 74 feet north-northeast of the northeast leg of the photo tower, and 25 feet west of the east edge of the clearing. The disk is a standard USC&GS reference disk set in a 12 by 12 inch concrete mark that is flush with the surface of the ground and is stamped KODAK MK 1.

Described by: R. L. Pfeu

Marked by: G. A. J.

Note:—The initial direction angles are in degrees, minutes, and seconds.

Refers to pages 108 and 109, Special Publication No. 120, or to pages 112 and 113, Special Publication No. 145. (To nearest meter only, when no trigonometric leveling is being done.)

M.S.

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Pacific Southwest Region

Station: KODAK STATE: Marshall Islands COUNTY: Eniwetok Atoll
 Chief: R. L. Pfeu YEAR: 1945 LOCALITY: Aniyaanii Island

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Reference mark No. 2 is 110.819 feet south-southeast of the station, 37 feet north of the south edge of the clearing, and 35 feet southwest of the southwest leg of the photo tower. The disk is a standard USC&GS reference disk set in a 12 by 12 inch concrete mark that is flush with the surface of the ground and is stamped KODAK MK 11.

The Aniyaanii photo tower is 21.425 meters southeast of the station. The tower is a 4-leg steel structure 25 feet in height. Distance and direction were taken from a stake with a nail in the center that is under the center of the tower.

M.S.

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 DESCRIPTION OF TRIANGULATION STATION

NAME OF STATION: RUNIT

STATE: Marshall Islands COUNTY: Eniwetok Atoll

CHIEF OF PARTY: R. L. Pfau

YEAR: 1948 LOCALITY: Runit Island

Surface-station mark, Note,* 1

DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND

Underground-station mark, Note,* 7

Reference mark, No. 1 Note,* 1.8

OBJECT	DISTANCE	DIRECTION	AZIMUTH
SAND U.S.N. 1948	meters	0° 00' 00.0	
reference mark No. 1	12.520	180 12 25	
reference mark No. 2	12.550	278 37 39	

Reference mark, No. 2 Note,* 1.8

Azimuth mark, Note,*

Witness mark, Note,*

Height of light above station mark 6 meters.

Height of telescope above station mark 6 meters.

Detailed description: The station is located approximately 450 feet north of the south end of Runit Island, 60 feet east of the high water mark on the lagoon side of the island, and 130 feet west of the high water mark on the seaward side. The disk is a standard USC&GS station disk set in a 12 by 12 inch concrete mark that projects 2 inches above the ground, and is stamped RUNIT.

Reference mark No. 1 is 41.075 feet north-northwest of the station, 60 feet east of the high water mark of the lagoon, and 2 feet south of the southwest corner of the southernmost quonset hut on Runit Island. The disk is a standard USC&GS reference disk set in a 12 by 12 inch concrete mark that is flush with the ground and is stamped RUNIT.

Reference mark No. 2 is 48.062 feet east of the station, 90 feet west of the high water mark on the seaward side of the island, and 4 feet south of a quonset hut. The disk is a standard USC&GS disk set in a 12 by 12 inch concrete mark that is flush with the surface of the ground, and is stamped RUNIT.

Described by G.R.S. Marked by G.A.J.

NOTE.—The initial direction must be to main sea level.

Refers to pages 108 and 109, Special Publication No. 120, or to pages 112 and 113, Special Publication No. 145.
 If nearest water only, when trigonometric leveling is being done.

~~SECRET~~
 TRAVERSE
 DESCRIPTION OF TRIANGULATION STATION

NAME OF STATION: ACOMON TRAVERSE STA.

STATE: Marshall Islands COUNTY: Eniwetok Atoll

CHIEF OF PARTY: R. L. Pfau

YEAR: 1948 LOCALITY: ACOMON

Surface-station mark, Note,* 1

DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND

Underground-station mark, Note,*

Reference mark, Note,*

Reference mark, Note,*

Azimuth mark, Note,*

Witness mark, Note,*

Height of light above station mark meters

Height of telescope above station mark meters

Detailed description: The station is located in the southwestern corner of Acomon Island, about 500 feet northeast of the southwest corner of the island, 200 feet northwest of the northwest corner of the causeway, and 120 feet east of the high-water mark of the lagoon. The disk is a standard USC&GS triangulation disk set in a 12 by 12 inch concrete mark that projects 2 inches and is stamped ACOMON TRAVERSE STA.

Described by G.R.S. Marked by G.A.J.

NOTE.—The initial direction must be to main sea level.

Refers to pages 108 and 109, Special Publication No. 120, or to pages 112 and 113, Special Publication No. 145.
 If nearest water only, when trigonometric leveling is being done.

~~SECRET~~
DESCRIPTION OF TRIANGULATION STATION

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 525
Rev. Oct., 1932

NAME OF STATION: BIJIRI TRAVERSE STA. STATE: Marshall Islands COUNTY: Eniwetok Atoll
CHIEF OF PARTY: Ralph L. Pfau YEAR: 1948 LOCALITY: Bijiri Island

Surface-station mark, Note
Underground-station mark, Note
Reference mark, Note
Reference mark, Note
Azimuth mark, Note
Witness mark, Note
Height of light above station mark, meters.
Height of telescope above station mark, meters.

DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND

DISTANCE	DIRECTION	AZIMUTH

Detailed description: The station is located in the southwestern corner of Bijiri Island, about 550 feet northeast of the southwestern tip of the island, 180 feet north of the south end of the island, and 110 feet east of the high-water mark of the lagoon. The disk is a standard USC&GS triangulation disk set in a concrete mark that projects 2 inches and is stamped BIJIRI TRAVERSE STA.

Described by: G.A.S.

Marked by: G.A.J.

NOTE:--The initial direction in this report should be read in conjunction with pages 108 and 109, Special Publication No. 120, or to pages 108 and 109, Special Publication No. 145. (See also page 108, when trigonometric leveling is being done.)

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~~SECRET~~
DESCRIPTION OF TRIANGULATION STATION

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 525
Rev. Oct., 1932

NAME OF STATION: RUNIT TRAVERSE STA. STATE: Marshall Islands COUNTY: Eniwetok Atoll
CHIEF OF PARTY: R. L. Pfau YEAR: 1948 LOCALITY: Runit Island

Surface-station mark, Note
Underground-station mark, Note
Reference mark, Note
Reference mark, Note
Azimuth mark, Note
Witness mark, Note
Height of light above station mark, meters.
Height of telescope above station mark, meters.

DISTANCES AND DIRECTIONS TO REFERENCE MARKS AND PROMINENT OBJECTS WHICH CAN BE SEEN FROM THE GROUND

DISTANCE	DIRECTION	AZIMUTH

Detailed description: The station is located in the central part of Runit Island, about 1200 feet northeast of the tip of the island, 100 feet above the larger sections of the island, 221 feet west of the high-water mark of the lagoon, and 121 feet east of a pontoon, 1 mile long, on the west side of the island. The disk is a standard USC&GS disk set in a concrete mark that projects 2 inches above the ground and is stamped RUNIT TRAVERSE STA.

Described by: G.A.S.

Marked by: G.A.J.

NOTE:--The initial direction in this report should be read in conjunction with pages 108 and 109, Special Publication No. 120, or to pages 108 and 109, Special Publication No. 145. (See also page 108, when trigonometric leveling is being done.)

RECOVERY NOTE TRIANGULATION STATION

R

NAME OF STATION SOUTH BASE STATE MARSHAL ISLANDS CENTRE ENIWETOK ATOLL
ESTABLISHED BY U.S.S. BOWDITCH 1944
RECOVERED BY *R.L.Pfau 1948

Detailed statement as to the fitness of the station for use. This station has been destroyed

J.C.H.

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~~SECRET~~

* Name of the report should be given in full, including the title and the name of the author, and the name of the agency to which the report was submitted.
U. S. GOVERNMENT PRINTING OFFICE: 1944 O-544444 16-26438-1

RECOVERY NOTE TRIANGULATION STATION

R

NAME OF STATION ENIWETOK ASTRO STATE MARSHAL ISLANDS CENTRE ENIWETOK ATOLL
ESTABLISHED BY U.S.S. BOWDITCH 1944
RECOVERED BY * R.L.Pfau 1948

Detailed statement as to the fitness of the station for use. This station has been destroyed

J.C.H.

~~SECRET~~

* Name of the report should be given in full, including the title and the name of the author, and the name of the agency to which the report was submitted.
U. S. GOVERNMENT PRINTING OFFICE: 1944 O-544444 16-26438-1

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43A

RECOVERY NOTE, TRIANGULATION STATION

R

NAME OF STATION SAND BAY DISK
ESTABLISHED BY
RECOVERED BY * R. L. Prall

Islands (Group) Eniwetok Atoll

Detailed statement as to the nature and condition of the station was recovered and found to be in good condition. A complete description follows:

The station is located on the third sand bar south of Runit Island, about 450 feet south of the northern end of the island and 100 feet east of the high-water mark on the lagoon beach. The disk is a standard USN triangulation survey disk set in an 8 by 8 inch concrete mark that projects 3 inches above the surface of the ground and is not stamped.

No reference marks were established at this station.

~~OFFICIAL USE ONLY~~

~~SECRET~~

* Name of the person to whom the station was returned should appear at the end of the recovery note.

U. S. GOVERNMENT PRINTING OFFICE

1944 O-348-1

Horton P. Stude

15-26488-1

RECOVERY NOTE, TRIANGULATION STATION

R

NAME OF STATION NORTH PASS DISK 1944
ESTABLISHED BY
RECOVERED BY * R. L. Prall

Islands (Group) Eniwetok Atoll
Runit Island

Detailed statement as to the nature and condition of the station was recovered and found to be in good condition. A complete description follows:

The station is located on the north end of Runit Island, 180 feet southeast of the north tip of the main island, 120 feet west of the northwest leg of the photo tower, and 35 feet east of the high-water mark on the lagoon side of the island. The disk is a standard USN triangulation survey disk set in an 8 by 8 inch concrete mark that projects 3 inches above the surface of the ground. There is no stamping on the disk.

No reference marks were established at this station.

~~SECRET~~

* Name of the person to whom the station was returned should appear at the end of the recovery note.

U. S. GOVERNMENT PRINTING OFFICE

1944 O-348-1

Horton P. Stude

15-26488-1

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Page 10

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Handwritten title or reference

CONTENTS

- (1) Index summary of the revised features of the tripartite release, less the expansion figures.
- (2) Base line of 1964.
- (3) Comparison of base line.
- (4) Base line production completed.
- (5) Abstract of the levels and distribution of industrial production.
- (6) Lists of Member countries and examples.
- (7) Report on identification of the counter order inventories: W-1, W-2, W-3, and W-4, and related type W-5 (1964-1965).

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TRIANGULATION

A scheme of first-order triangles composed of check figures was executed along the eastern coast of the island from a first-order base line on Engebi Island. This scheme extends northward to Engebi Island and a traverse to Aniyani Island. It was executed for the purpose of reactivating local stations on the activated islands and to establish distances and bearings between certain installations.

All observations were made at night and standard procedure was followed throughout. The maximum triangle closure for the entire scheme was 01.10 seconds and the average 01.00 seconds. The maximum triangle closure for the base stationing was 01.10 seconds and the average 01.00 seconds.

With the exception of station NARA, which could not be occupied because of excessive wind, all stations were occupied. Traverse ties were run to the perimeter lights towers on Engebi, Acomon, Funii, and Aniyani Islands.

In order to compare the accuracy of the U.S.S. BOWDITCH with the new survey stations NARA, 1944 and SAND USN, 1944 were incorporated into the scheme.

BASE LINE

A first-order base line was established on Funii Island between stations NARA, 1944 and the established station RUNIT. The configuration of the island necessitated the adoption of a broken base consisting of four sections of varying lengths. First-order invert ties were run and standard procedure was followed throughout.

The combined possible closure of the total measurement is 1 part in 2,100,000.

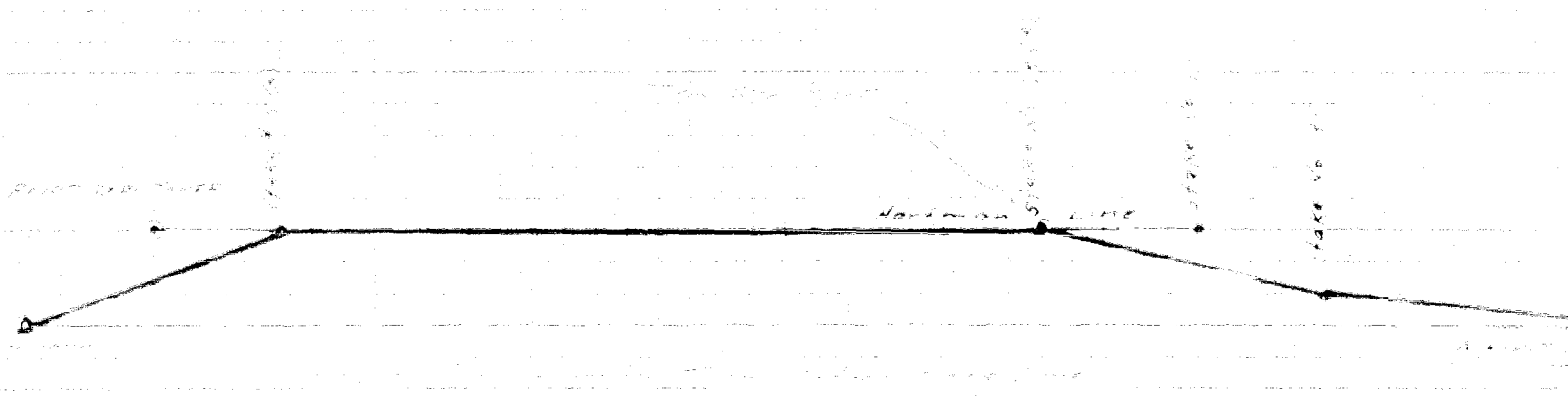
This base line is comparable to a second-order base line measured on the same island by the U.S.S. BOWDITCH in 1944, due to the fact that a considerable portion of the island extending off the south end of the island has eroded away.

The base line station NARA was located through station CORAL, a newly established station located in the same general area as station NARA, 1944, but not connected with that station. This is the most convenient position at which the construction of a station suitable for triangulation was possible.

The base stationing was run in accordance with the specifications for first-order triangulation in the U.S. Manual. The figure is larger than is generally required for first-order triangulation. This is the only respect in which the detailed specifications for first-order specifications, other than the number of triangles, exceeded the intent of the scheme.

~~SECRET~~

BASE LINE SKETCH
(No. 51010)



Note: The Hartman line was staked out first, using regular
sequence of stake numbers thru stake No 33.
The portion of this line from stake No 4 thru stake No 28
was also used as a section of the Triangulation Base
line and stakes 4 + 28 of the Hartman line were designated
4(A) and 28(B) to indicate that they were angle stations
in the Base line. Following the same system of designating
angle stations C of the base line was designated 9(C), 9 being
the stake No. The designations were used in all record
book + computations.

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	OBSERVED	COMPUTED	DIFFERENCE
N.B	394.0010	394.0010	0.0000
A	160.0000	160.0000	0.0000
B	160.0000	160.0000	0.0000
C	174.0000	174.0000	0.0000
RUNIT	0.0000	0.0000	0.0000
E	0.0000	0.0000	0.0000
N.B - A	234.0010	234.0010	0.0000
N.B - B	234.0010	234.0010	0.0000
N.B - C	220.0010	220.0010	0.0000
A - B	0.0000	0.0000	0.0000
A - C	18.0000	18.0000	0.0000
B - C	11.0000	11.0000	0.0000
C - RUNIT	0.0000	0.0000	0.0000
RUNIT - A	160.0000	160.0000	0.0000
RUNIT - B	160.0000	160.0000	0.0000
RUNIT - C	174.0000	174.0000	0.0000
RUNIT - E	0.0000	0.0000	0.0000
			369.6456
			+ 7.2195
			1199.9266
			+ 87.0644
			41.4843
			+ 9.7136
			580.9124
			- 0.0164
			2591.9689
			Log = 3.4136298

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 685

ABSTRACT OF WYE LEVELS
AND

COMPUTATION OF INCLINATION CORRECTIONS

(UNIT I)

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Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	REMARKS	MEAN ELEVATION	REMARKS
	Meters	Meters	Seconds		Meters	
Mark N. Base USN 1944 Bench						
N. Base USN 1944	0 ⁰	11.0000	0.0			
N. Base Setup	2984	11.0000	1.2			
1	50 ⁰	11.0000	0.0			
2	50 ⁰	11.0000	0.0			
2 + 25	25 ⁰	11.0000	0.0			
3	50 ⁰	11.0000	0.0			
4	50 ⁰	11.0000	0.0			
5	50 ⁰	11.0000	0.0			
6	50 ⁰	11.0000	0.0			
4(A)	50 ⁰	11.0000	0.0			
			15.0			

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U. S. COAST AND GEODETIC SURVEY
Form 635

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ABSTRACT OF WYE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

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Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTIONS	ELEVATION	MEAN ELEVATION	REMARKS
		Meters	Seconds	Meters	Meters	
4 (A)						
5	50	0.0000	0.0			
6	50	0.0000	0.0			
7	50	0.0000	0.0			
8	50	0.0000	0.0			
9	50	0.0000	0.0			
10	50	0.0000	0.0			
11	50	0.0000	0.0			
12	50	0.0000	0.0			
13	50	0.0000	0.0			
14	50	0.0000	0.0			
15	50	0.0000	0.0			
16	50	0.0000	0.0			
17	50	0.0000	0.0			
18	50	0.0000	0.0			
19	50	0.0000	0.0			
20	50	0.0000	0.0			
21	50	0.0000	0.0			
22	50	0.0000	0.0			
23	50	0.0000	0.0			
24	50	0.0000	0.0			
25	50	0.0000	0.0			
26	50	0.0000	0.0			
27	50	0.0000	0.0			
28 (B)	50	0.0000	0.0			

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 635

~~SECRET~~
ABSTRACT OF WYE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

Reproduced from the holdings of the National Archives
Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	<i>Meters</i>	<i>Meters</i>	<i>Secs</i>	<i>Meters</i>	<i>Meters</i>	
28 (B)						
1	175	0.18	0.1			
2	175	1.60	2.0			
3	175	0.08	0.1			
4	175	0.60	0.8			
5	175	0.20	0.6			
6	175	0.10	0.6			
7	175	1.46	2.2			
8	175	1.52	2.1			
9 (C)	175	0.17	0.2			
		5	7.1			

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ABSTRACT OF WYE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

Reproduced from the holdings of the National Archives
Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
9 (C)						
1	1000	1.000	0.000			
2	2000	2.000	0.000			
3	3000	3.000	0.000			
4	4000	4.000	0.000			
5	5000	5.000	0.000			
6	6000	6.000	0.000			
6+25	2500	2.500	0.000			
7	7000	7.000	0.000			
8	8000	8.000	0.000			
9	9000	9.000	0.000			
10	10000	10.000	0.000			
11	11000	11.000	0.000			
Bench Δ Run 1	12000	12.000	0.000			
Mark Δ Run 2	13000	13.000	0.000			

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SECRET

Station NORTH BASE, U.S.N. 1944 State Marshall Is. Eniwetok Atoll
 Chief of party Ralph L. Pfau Dec 30 January 1944 Computed by R.L.P.
 Observer G.R. Strobe Instrument Repeata #4 237 Checked by R.E.B.
-188

11-9503

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 Pacific Southwest Region

OBSERVED STATION	Observed direction	Eccentric reduction	Refraction correction	Corrected direction with zero initial	Adjusted direction*
4 (A)				00.00	
UNIT, U.S.C.S.S. 1948					
No final instrument correction					

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* These columns are for office use and should be left blank

~~SECRET~~
LIST OF DIRECTIONS

~~OFFICIAL USE ONLY~~

Station 4 (A)

State Marsau Is. Atoll

Chief of party Ralph P. ...

Date 24 Jan 1954

Computed by R.L.P.

Observer G.R. Stroe

Instrument H-334

Checked by R.L.P.

MAR

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Pacific Southwest Region

OBSERVED STATION	Observed direction	Level reduction	Corrected direction with zero initial	Adjusted direction*
<u>28 (B)</u>	<u>000 0000</u>		<u>0 00 00.00</u>	
<u>North Base, US A. ...</u>	<u>110 56 21</u>			

at least within ...

~~SECRET~~

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~~SECRET~~

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 24A
Rev. Oct., 1932

Station 28(B) State Marshall Is. Eniwetok Atoll

Chief of party Ralph L. Pfaender Date 29 Jan. 1943

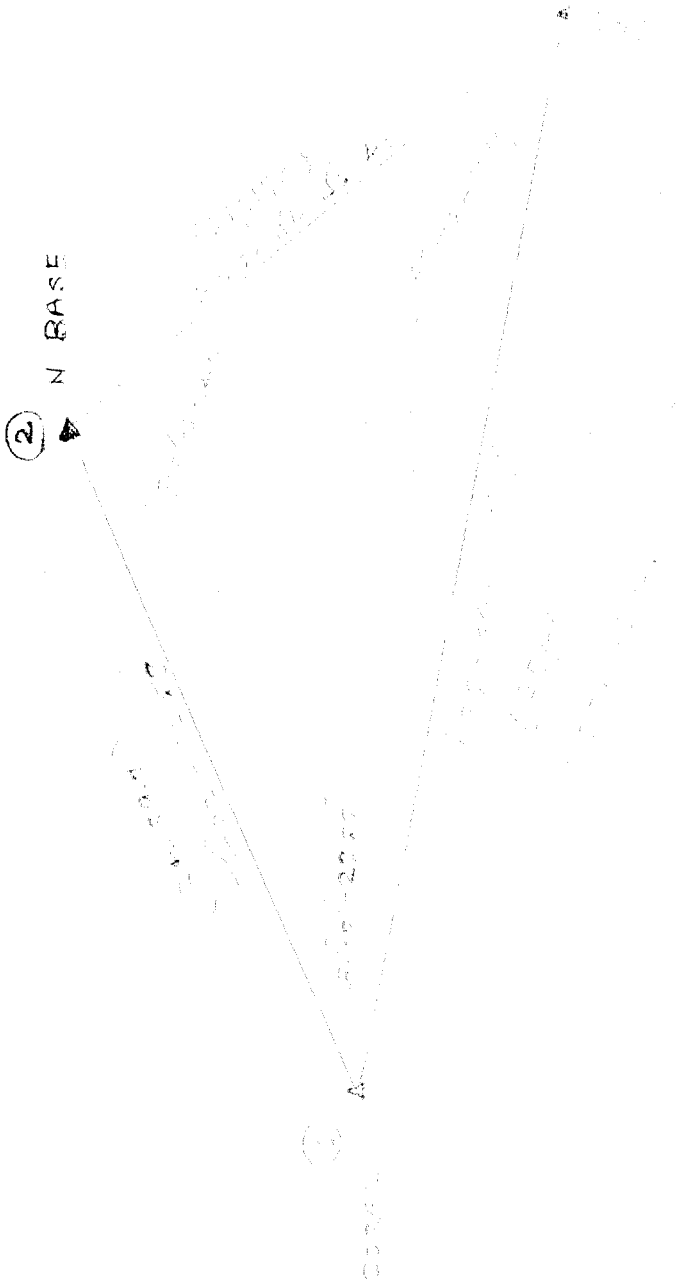
Computed by R. L. P.

Observer G. D. Clark

R. L. P.

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Adjusted
direction*



53

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U. S. COAST AND GEODETIC SURVEY
Form 24A
Rev. Oct., 1932

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LIST OF OBSERVATIONS

Station 28(B)

State Marshall Is. Eniwetok Atoll

Chief of party Ralph L. Pton

Date 29 Jan 1946

Computed by R.L.P.

Observer G.R. Strode

Instrument Zepeda 314 Checked by R.L.P.

GOVERNMENT PRINTING OFFICE: 1945 11-9503

OBSERVED STATION	Case and direction	Eccentricity of station	Semi-axis of station	Corrected direction with zero initial	Adjusted direction*
9 (C)				000 00.00	
4 (A)					

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LIST OF DIRECTIONS

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 24-A
Rev. Oct., 1932

Station 9 (C)

State Marshall Is. Eniwetok Atoll

Chief of party Ralph P. ...

Date 29 January 1948

Computed by R. L. P.

Observer G. R. Strove

Instrument Repeater # H-334

Checked by R. L. P.
948

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OBSERVED STATION	Observed direction	Index in seconds	Sea level reduction*	Corrected direction with zero initial	Adjusted direction*
<u>Runit, U.S. CGS. 28 (B)</u>	<u>100 00.00</u>			<u>0 00 00.00</u>	
	<u>130 11 0 42</u>				

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~~SECRET~~
 LAST OF DIRECTIONS

Station RUNIT, U.S.C.G.S 1944 State Marshall Is - Eniwetok Atoll
 Chief of party Ralph L. Ffau Date 24 Jun. 1952 Computed by R.L.P.
 Observer G.R. Strode Instrument T. Repeater # 334 Checked by R.L.P.
= 298

OBSERVED STATION	Observed direction	Magnetic reduction	True level reduction	Corrected direction with zero initial	Adjusted direction*
<u>North Base, U.S.N. 1944</u>	<u>00 00 00.00</u>				
<u>9</u>					
<i>No other observations recorded</i>					

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UNITED STATES DEPARTMENT OF COMMERCE
WASHINGTON

UNITED STATES DEPARTMENT OF COMMERCE
WASHINGTON

National Bureau of Standards

National Bureau of Standards

Certificate

Certificate

10 METER IRON NICKEL ALLOY TAPE

10 METER IRON NICKEL ALLOY TAPE

Linear Expansion Coefficient

Linear Expansion Coefficient

NBS No. 322

NBS No. 322

U.S.C. & G.S. No. 922

APPROVED

United States Coast & Geodetic Survey,
Washington, D. C.

United States Coast & Geodetic Survey,
Washington, D. C.

This tape has been compared with the standards of the United States under a horizontal tension of 15 kilograms. The interval (0 to 50 meters) has the following length at 25 °C under the conditions given below:

This tape has been compared with the standards of the United States under a horizontal tension of 15 kilograms. The interval (0 to 50 meters) has the following length at 25 °C under the conditions given below:

Supported at the 0, 25, and 50 meter points 49.99748 meters

Supported at the 0, 25, and 50 meter points 49.99748 meters

Supported at the 0, 12.5, 25, 37.5, and 50 meter points, with the 12.5 and 37.5 meter points 6 inches above the plane of the 0 and 50 meter supports 49.99712 meters

Supported at the 0, 12.5, 25, 37.5, and 50 meter points, with the 12.5 and 37.5 meter points 6 inches above the plane of the 0 and 50 meter supports 49.99712 meters

Thermometer weighing 45 grams were attached at points 1 meter inside the terminal marks

Thermometer weighing 45 grams were attached at points 1 meter inside the terminal marks

These comparisons were made on the section of the tape near the edge of the tape marked with a small "a" or "v" or dots near the graduation

These comparisons were made on the section of the lines near the end on the edge of the tape marked with a small "a" or "v" or dots near the graduation

The values for the lengths are not in error by more than 1 part in 500,000; the probable error does not exceed 1 part in 1,500,000

The values for the lengths are not in error by more than 1 part in 500,000; the probable error does not exceed 1 part in 1,500,000

The values for the lengths were obtained from measurements made at 22.6 °C, and in reducing to 25 °C, the thermal expansion of 40,055 millimeter per 10 meters per degree centigrade was used

The values for the lengths were obtained from measurements made at 22.6 °C, and in reducing to 25 °C, the thermal expansion of 40,055 millimeter per 10 meters per degree centigrade was used

The weight per meter of the tape previously determined is 15.6 grams

The weight per meter of the tape previously determined is 15.6 grams

Lewis V. Judson
Chief Length Section
Metrology Division

For the Director,

Lewis V. Judson
Chief Length Section
Metrology Division

X-12-100K 2.17114495
Test completed October 14, 1953

U.S. GOVERNMENT PRINTING OFFICE: 1953

UNITED STATES DEPARTMENT OF COMMERCE
WASHINGTON

National Bureau of Standards

Certificate

10 METER IRON NICKEL ALLOY TAPE

Linear Expansion Coefficient

NBS No. 324

U.S.C. & G.S. No. 922

APPROVED

United States Coast & Geodetic Survey,
Washington, D. C.

This tape has been compared with the standards of the United States under a horizontal tension of 15 kilograms. The interval (0 to 50 meters) has the following length at 25 °C under the conditions given below:

Supported at the 0, 25, and 50 meter points 49.99652 meters

Supported at the 0, 12.5, 25, 37.5, and 50 meter points, with the 12.5 and 37.5 meter points 6 inches above the plane of the 0 and 50 meter supports 49.99616 meters

Thermometer weighing 45 grams were attached at points 1 meter inside the terminal marks

These comparisons were made on the section of the tape near the edge of the tape marked with a small "a" or "v" or dots near the graduation

The values for the lengths are not in error by more than 1 part in 400,000; the probable error does not exceed 1 part in 1,500,000

The values for the lengths were obtained from measurements made at 22.6 °C, and in reducing to 25 °C, the thermal expansion of 40,052 millimeter per 10 meters per degree centigrade was used

The length per meter of the tape previously determined is 15.6 grams

Lewis V. Judson
Chief Length Section
Metrology Division

X-12-100K 2.17114495
Test completed October 14, 1953

~~OFFICIAL USE ONLY~~

~~SECRET~~

~~SECRET~~

U. S. DEPARTMENT OF COMMERCE
WASHINGTON

National Bureau of Standards

Certificate

FOR

30-Meter Steel Tape
NBS No. 8305

RECEIVED OCT 6 1947

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Maker's Identification Mark
Lufkin Rule Co.
C. & G. S. No. 3661

SUBMITTED BY

United States Coast & Geodetic Survey,
Washington 25, D. C.

This tape has been compared with the standards of the United States. It complies with the specifications for a standard tape, and the intervals indicated have the following lengths at 68° Fahrenheit (20° centigrade) under the conditions given below:

Supported on a horizontal flat surface:

Tension: 4 1/2 Kilograms

Interval Length
(0 to 30 meters) 30.0017 meters

Tension: 5 Kilograms

Interval	Length	Interval	Length
(0 to 1 meter)	1.0001 meters	(0 to 16 meters)	16.0006 meters
(0 to 2 meters)	2.0002 "	(0 to 17 meters)	17.0010 "
(0 to 3 meters)	3.0003 "	(0 to 18 meters)	18.0007 "
(0 to 4 meters)	4.0005 "	(0 to 19 meters)	19.0009 "
(0 to 5 meters)	5.0004 "	(0 to 20 meters)	20.0008 "
(0 to 6 meters)	6.0000 "	(0 to 21 meters)	21.0007 "
(0 to 7 meters)	7.0007 "	(0 to 22 meters)	22.0008 "
(0 to 8 meters)	8.0006 "	(0 to 23 meters)	23.0010 "
(0 to 9 meters)	9.0005 "	(0 to 24 meters)	24.0010 "
(0 to 10 meters)	10.0005 "	(0 to 25 meters)	25.0008 "
(0 to 11 meters)	11.0001 "	(0 to 26 meters)	26.0012 "
(0 to 12 meters)	12.0005 "	(0 to 27 meters)	27.0008 "
(0 to 13 meters)	13.0002 "	(0 to 28 meters)	28.0012 "
(0 to 14 meters)	14.0009 "	(0 to 29 meters)	29.0015 "
(0 to 15 meters)	15.0007 "	(0 to 30 meters)	30.0017 "

Test No. 2.1/113718

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The comparisons of this tape with the United States Standard Steel Tape were made at a temperature of 24.9° Centigrade and in reducing to 68° Fahrenheit (20° centigrade). The coefficient of expansion of the tape is assumed to be 0.00000645 per degree Fahrenheit (0.0000116 per degree centigrade).

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Tape Certificate Continued NBS No. 8305

When supported as indicated below:

ension: 5 kilograms

Points of Support	Interval	Length
0 and 5 meters	(0 to 5 meters)	5.0003 meters
0 and 10 meters	(0 to 10 meters)	10.0002 "
0 and 15 meters	(0 to 15 meters)	14.9997 "
0 and 20 meters	(0 to 20 meters)	19.9989 "
0 and 25 meters	(0 to 25 meters)	24.9971 "
0 and 30 meter	(0 to 30 meters)	29.9953 "

The above values for the lengths are not in error by more than 0.0002 meter.

The comparison of this tape with the Bench Standards were made at the center of the ends of the lines farthest from the observer when the zero of the tape was at his left hand.

The weight comparison of this tape was found to be 12.0 grams.

For the Director,

Lewis V. Judson

Lewis V. Judson,
Chief, Length Section,
Metrology Division

Test No. 2.1/113715
Test completed: Sept. 27, 1947

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V A. GENERAL METHOD ADAPTED FOR ESTABLISHMENT OF
OF STAKES FOR INSTALLATION OF ALL ISLANDS.

The general method adapted for establishing stakes on all islands consists of the precise measurement of one line, using a first-order aneroid barometer standard base line procedure, and the establishment of other points by normal or chord off-sight from this line.

Since tapes are not attached to the stakes for the forward and backward measurement, the tape ends of the forward measurement of the line were semi-permanently marked on the stakes to be used as a proportional correction applied to distances as determined in the forward measurement of the magnitude and distance to make the resulting total length of line equal to the sum of the forward and backward measurements.

In all cases, one of the Azimuths (Nathan, Gamma or Timing) radiating from the zero base line was selected for precise measurement. Which of the three lines was selected was dictated by local conditions on each island.

Although the Dugout line was the most economical line to measure, since the principal number of stations could be established by direct sight or by triangulation from the base line stakes, and necessitated the least amount of pre-computing, it was possible to measure this line on only one of the islands.

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Since there is but little contact between the AEC lines on any of the islands, the method used was well adapted to the purpose, eliminating the necessity for the measurement of all lines, and also the possibility of a systematic error that would have been present in the direct measurement of all lines with interior tapes and methods.

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On Engebi Island, the line established by standard base line procedure was the Zero Tower triangulation station ELGIN line. This was the only one of the REC lines which was not obstructed at the time. The probable error of the measurement was 1 part in 270,000.

The timing station is located on this line, and the timing station stake was established by sighting from stake No. 23 of the line.

The Gamma A and Gamma B station stakes were established by chord off-sets from the Zero Tower - Elgin line.

Stakes for the blast footing, including the footing at 3900 feet from Zero were established by chord off-sets from the Zero Tower - Elgin line. The blast footing stakes at 4200 and 4600 feet from Zero were established by prolonging the line beyond the 3900 foot stake. Distances beyond the 3900 foot stake were established by use of a 300 foot steel tape.

The centerline of the 80. YAL runway was selected so as to run diagonally across the air strip, thus affording a clear line of sight. From the Zero Tower a traverse was run along the centerline, and stakes set flush with the runway surface at the required distances from Zero, and chord off-set stakes set on either side of the runway.

Measurements were made with a 300 foot steel tape.

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The orientation of the centerline was determined by an observed angle at the Zero Tower from the Bu. Y&D centerline and triangulation station.

A line of reference rods was laid out in front of each line of Bu. Y&D units and two reference measurements made to each of two marked points on each rod.

The 3000 foot northward stake on the Bu. Y&D was selected as determining the centerline in the U. S. units. With the instrument set over this point and oriented back to Zero a traverse was run back to the Zero and stakes set at the required intervals.

A 300 foot steel tape was used for these measurements. The azimuth of the centerline is obtained by previously computed Bu. Y&D angles.

The O.C.E. building stake at 1000 feet from Zero was established by direct measurement of angle and distance from the Zero Tower triangulation mark.

The most northern body O.C.E. building stakes at 1500 and 2500 feet from Zero were established by known off-sets from the Bu. Y&D centerline. The western and westerly stakes at these distances were established by the off-sets from the northeasterly stake. A 300 foot steel tape was used for these measurements.

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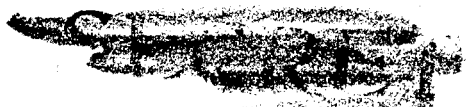
Four reference stakes were set in each of the C.C.E. buildings, with the exception of the type "B" structure at 2500 feet from the Zero Tower. Measurements made to drill holes in eight reinforcing rods were project horizontally from the building. Four of these rods are on the front face of the building (two near the base and two near the top), and four are similarly located on the back of the buildings.

All reference measurements in the C.C.E. buildings and to Bu. Y&D units are slope measurements and were made with steel tape No. URSAC 5874. This tape was left in the area for use in making post-operation measurements.

In order to number the multiple C.C.E. buildings with reference to points which will not be destroyed, a traverse line was run from the Zero Tower to the 2500 foot stake of the Bu. Y&D centerline. Measurements were made and reference stakes set near each of the C.C.E. buildings, and back to the Zero Tower. The traverse stakes were in turn connected to the reference stakes at each building. This traverse was measured with a 300 foot steel tape.

Range pole No. 2 was located at a normal off-set from the Zero Tower - B.M. line. Range poles No. 2 and No. 3 were similarly located, but were later moved. Range pole No. 3 was moved 40 feet northward and the line normal to the original line of poles. Range pole No. 2 was moved 20 feet northeastward and the normal to the original line of poles. Thus the poles are still on a straight line, but the line is no longer parallel to the Zero - Timing Station line.

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The station ~~was~~ located by ~~Dr. Clark~~, as selected by
Dr. Clark of the ~~Army~~, by observed angle and
distance from ~~the~~ ~~triangulation~~ station ~~EDWIN~~.

The ~~width~~ ~~was~~ located by ~~the~~ ~~observed~~ angle and distance
from triangulation station ~~EDWIN~~.

The ~~tank~~ ~~was~~ ~~located~~ by ~~the~~ ~~observed~~ angle and
distance from ~~the~~ ~~triangulation~~ station ~~EDWIN~~ of range
pole No. 3.

A line of ~~level~~ ~~was~~ ~~run~~ ~~along~~ ~~the~~ ~~triangulation~~ station ~~EDWIN~~ for a distance
of 500 feet or ~~more~~ ~~of~~ ~~the~~ ~~range~~ ~~pole~~. This line
is tied in to triangulation station ~~EDWIN~~ and other points
in the vicinity of ~~the~~ ~~triangulation~~ station ~~EDWIN~~.

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Since the base line extended from Roanoke Island to Rojoa Island, and with the aid of the aid of obstructions, it was selected at the time the line was by standard base line procedure.

This line extended over land, making it necessary to carry the distance from Rojoa Island by triangulation.

This was accomplished by means of a short auxiliary base line - measured with leveling staffs - standard base line procedure - on a level and level island.

The probable error of the base line, including sections determined by triangulation, does not exceed 1 part in 850,000. The probable error is stated in this manner because in its construction it was necessary to use the two determinations of distance across each water gap one of which distance is determined from a geometrically stronger figure than the other. In each case only the distance determined from the stronger figure was used as the distance across the gap, the weaker determination being used as a check only.

Gamma stations A, B, and C were located by set ups or set backs from stations of the base line. Gamma A was set at 2130 feet from zero, station B was the maximum distance obtainable on Rojoa Island.

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The blast footing stakes were established by chord off-sets from the measured Gamma line.

The Timing station stake was established by chord off-set from the 690 foot blast footing stake.

The range pole stakes were originally set by off-sets from the Zero Tower - Timing Station line and at the positions specified in the NE drawings. Range pole No. 1 was set in the specified position, but range poles No. 2 and No. 3 were not set in accordance with the drawings. This change was made by the resident engineer because the pole No. 3 fell in front of the wind base which had been spotted in by eye prior to the arrival of the survey party on the island.

The positions of range pole No. 2 and No. 3 as actually set were determined by measurements and distances from stakes of the measured Gamma line. The poles as set are on a straight line, but the line is not parallel to the Zero Tower - Timing Station line.

Stakes for the computation of the M.P. units at each specified distance from zero were established by chord off-sets from the Gamma line. However, in starting this report Lieut. Colonel John Krichbaum, USMC, advises that all of these stakes have been destroyed or removed. The section of computations relating to the M.P. for the stations on Anon, Blijiri and Nejae military installations are of no significance, and should be disregarded.

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The wind was, but will be, bank revetment which were in the process of construction when the survey party arrived on the island were picked up. Measured angles and distances from fixed stake holes were taken on a line.

The photo tower was located by measured angle and distance from triangulation station Grand...

The causeway centerline was established by offset from the marker line run for the purpose of orienting the AEC installation to the island, it being the only line available at the time the work was required.

One causeway centerline stake was set above high water line on Aomon I. and another on Bijiiri I. The distance between these stakes was determined by triangulation from an auxiliary base line measured with a 100 foot steel tape. Additional centerline and pile line stakes were later set on both Aomon and Bijiiri Islands.

A bench mark was established on Bijiiri Island ~~XXXXX~~ ~~XXXXXX~~ the elevation of which was dependent upon an assumed elevation of 100 feet for the high water line on the beach. This bench mark was 7.0 feet for the Aomon bench mark. A secondary bench mark was established on Bijiiri I., and connected by spirit level to the bench mark on Aomon I.

Pile centerline and grade elevations were determined by leveling from these bench marks, and a line of the centerline and pile line stakes.

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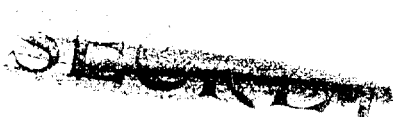


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The following description is based on the profile
interior view of the runway centerline and on the ground profile
profile.

Pre-grading and construction level of runway between and
Bijiri Islands, use of the Gamma 100 as a centerline.
Rod readings were taken every 10 feet along the centerline,
and side shots taken at 100-foot intervals and at breaks in
grade for a distance of 100 feet on either side of the centerline.
These side shots were taken at 10-foot intervals along
the centerline, and a slope a line was drawn to the centerline.

[Faint, illegible text, possibly a signature or stamp]



02A

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On unit 11113 all of the lines were unobstructed and available for measurement. The Hartman line was selected as the line to be measured first because it was the most economical line for measurement from the standpoint of establishing stakes for the line to be measured because it was so situated that a large portion of it could be used as one section of the triangulation of the base line.

The Hartman line was measured in three sections. The first section extends from the benchmark station mark to stake 4(A). This section is also one section of the first-order base line extending from stake 4(A) to stake 28(B). The second section extends from stake 28(B) to stake 33. Stake 28(B) was later replaced by a standard USC&GS triangulation station disk set in a concrete post and stamped "PACIFIC SW. SURVEY".

The Hartman line from the benchmark to stake 33 has a computed probable error of 1 part in 2,275,000. The probable error of the triangulation line which was measured between station NORTH PASADENA, 1944 and newly established station 2002 is different from the section titled "BASE LINE".

The blast holes and stakes were located by set ups or set backs from stake 1 of the Hartman line measurement.

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The Timing station was located by chord off-set from the Hartman line.

The Gun a set 44 m were located by chord off-sets from the Hartman line.

The 1000 foot triangulation station was located by chord off-set from the 1000 foot point on the Water Tower Timing station line.

The range pole stakes were set by chord off-sets from the Hartman line. The distance from the line to the range pole stake by setting up a theodolite at the ground angle and distance computed for the chord off-set from the 1500 foot point on the Water Tower Timing station line to range pole stake No. 1. The range pole stake on a line parallel to the Hartman line.

The wind line was located by an observed angle and distance from station 10 of the first-order line. From this data the distance from station 10 to the tower was computed.

The tank reflector was located by an observed angle and distance from station 6 of the 2nd (B) to 2 (C) section of the first-order line.

The Photo Bench was located by an observed angle and distance from triangulation station No. 1000, 1944.

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A standard U.S.G.S triangulation mark stamped "TRAVERSE STA RUNIT" and set in a concrete post was established as a permanent mark to replace base mark (base 28(B)).

Pre-grading cross section levels were run on Runit Island using the center line as a centerline. Rod readings were taken every 50 feet along the centerline, and side shots taken at 100 foot intervals and breaks in grade for a distance of 300 feet out from the centerline. These side shots were taken at 100 foot intervals along the centerline, and along a line normal to the centerline.

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FORM NO. 10-62 (REV. 1-15-62)

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LARGEST ISLAND

LOCATION OF STRUCTURES FROM LIGHT TOWER

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STRUCTURE	DISTANCE FROM ZERO	AZIMUTH FROM ZERO
Timing Station	3900	323 39 12
Blast Footing	1400	310 27 53
" "	1700	" " "
" "	1800	" " "
" "	2100	" " "
" "	2400	" " "
" "	2700	" " "
" "	3000	" " "
" "	3300	" " "
" "	3600	" " "
" "	3900	" " "
" "	4200	" " "
" "	4800	" " "
Blast Building	4700	314 45 43
Gamma A Station	4250	314 01 41
" B "	4900	" " "
Ionization Sta.	3000	314 24 17
Ctr. of Line of Cables Bu Y&D	3050	262 48 22
" " "	3700	" " "
" " "	4200	" " "
" " "	4700	" " "
" " "	5000	" " "
" " "	3600	" " "

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STRUCTURE	DISTANCE FROM ZERO	AZIMUTH FROM ZERO *
O.C.E. Type A	1000	289 22 42
" " A	1100	281 33 51
" " B	1500	287 00 33
" " B	1500	279 44 01
" " C	1500	283 14 31
Range Pole # 1	1200	304 18 05
" " # 2	Distart from Pole #1 at 1000 ft. Az from Pole #1 312 53 22	
" " # 3	" " " " " " " " " " " "	
Tank Revetment	#3	348 ³⁴⁶ 56 ²⁹ 22 ¹⁹
Winch Base	411	311 02 32
Bu S Stakes	2200	257 02 48
" " "	2300	" " "
" " "	2400	" " "
" " "	2500	" " "
" " "	2600	" " "
" " "	2700	" " "
" " "	2800	" " "
" " "	2900	" " "
" " "	3000	" " "

* Except where state otherwise

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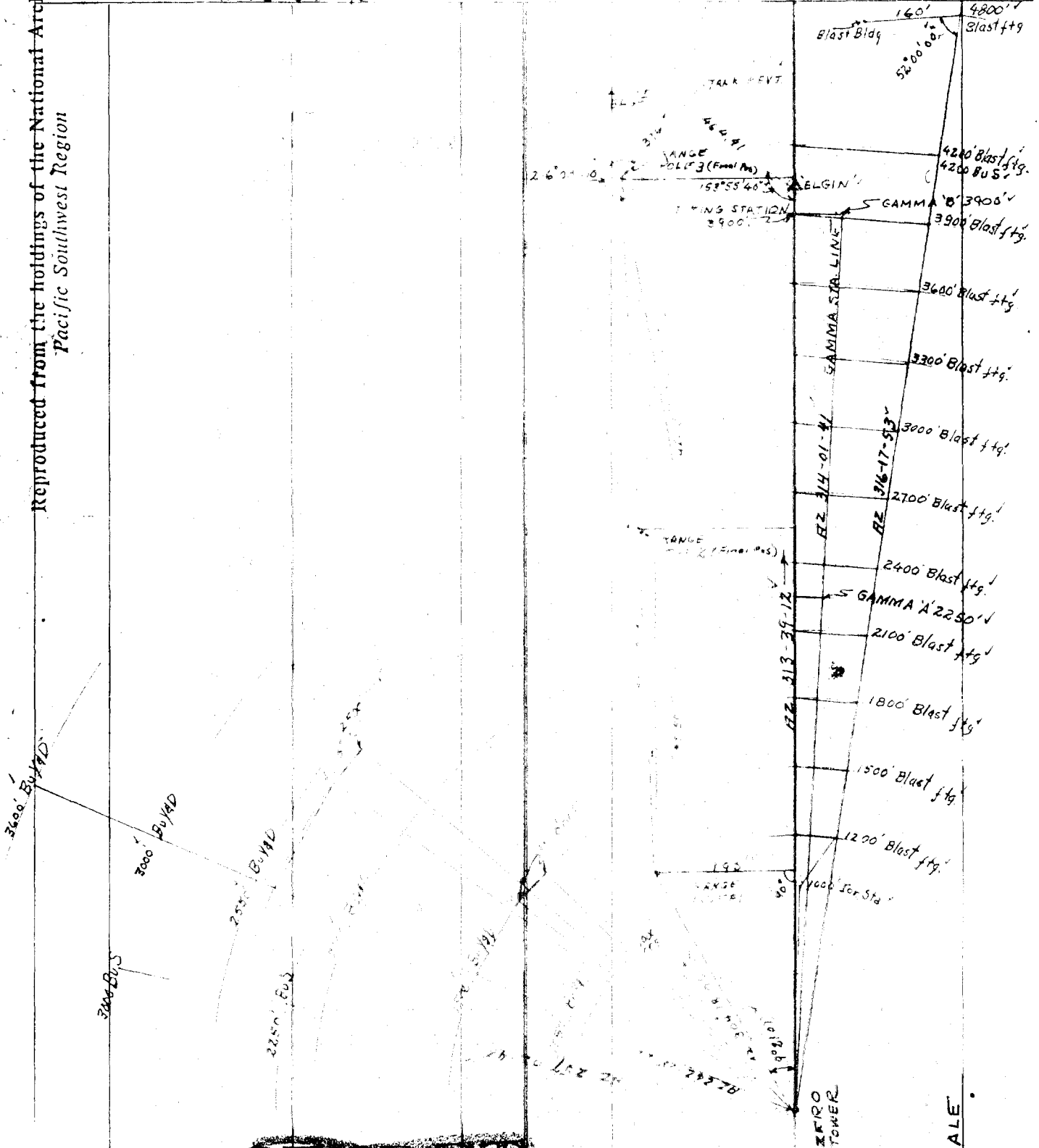
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ENGEBI

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COMPUTATIONS OF TRAVERSE BETWEEN
ENGINE ZERO TOWER AND TRIANGULATION
STATION BELGIN
DISTANCES TO STAKES OF LINE ABOVE
LINE USED IN ESTABLISHING OTHER
POINT.

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COMPUTATION OF

ENGERBI

TRAVERSE
BASE LINE

U. S. GOVERNMENT PRINTING OFFICE 22 135M

SECTION	DATE	DIR OF MEAS	TAPE NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP	COR.			RECTIONS			REDUCED LENGTH	ADOPTED LENGTH
					Tape Length	Meters		Temp	Tape and Category	Set up	Inclination	Sea level	Meters				
Bench ENGERBI ZERO	11-16-47	7	701	3	74	74.000	17.2	+0.0002	-0.0816	+0.0000	0.0000	0.0000	74.0000	74.0000			
To		4	720	2	72	72.000	17.2	+0.0002	-0.0817	+0.0000	0.0000	0.0000	72.0000	72.0000			
Bench A. E. ELGIN		4	700	2	70	70.000	17.2	+0.0002	-0.0816	+0.0000	0.0000	0.0000	70.0000	70.0000			

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70'

70'

70'

DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 538
Rev. April 1926

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COMPUTATION OF

TRAVERSE
BASE LINE

SECTION	DATE	DIR. OF OBS. INCLAS.	TAPE NO.	TAPE SUPPLY	UNCORRECTED LENGTH		TEMP	COR.		RECTIONS	REDUCED LENGTH	ADAPTED LENGTH	DISTANCE FROM ZERO TO ZERO
					Meters	Feet		Meters	Feet				
1. Bench FROM LEVELING TO	2/16/47	E	921				24.7		0.0238	0.0000			24.7
2.													
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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
REV. APRIL 1953

TRAVERSE
BASE LINE

COMPUTATION OF

EX. 6561

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SECTION	DATE	DIR. OF MEAS.	TAPE NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP.	COR.	RECTIONS			REDUCED LENGTH	ADJUSTED LENGTH	DISTANCE FROM ANGLE TO TOWER	
					Temp	Meters			Setback	Inclination	Sea level				Meters
EX. 6561															

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~~ABSOLUTE LEVELS~~
AND

COMPUTATION OF INCLINATION CORRECTIONS

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POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters	Seconds	Meters	Meters	
Mark ENGBELZRO				10.000 (10.000)		
Bench ENGBELZRO		+2.116				
1	50.0	+0.892	1.0			
2	50.0	+0.517	0.7			
3	50.0	+1.103	1.4			
4	50.0	+0.150	0.2			
5	50.0	+0.402	0.5			
6	50.0	+0.038	0.0			
7	50.0	+0.125	0.2			Σ Cor = 2.4 mm
8	50.0	+0.131	0.2			
9	50.0	+1.260	1.5			Σ Cor = 4.2 mm
10	50.0	+0.098	0.1			
11	50.0	+1.152	1.4			Σ Cor = 7.4 mm
12	50.0	+1.647	2.0			
13	50.0	+1.521	1.9			Σ Cor = 10.6 mm
13 + 25	50.0	+0.168	0.2			
13+25 Setup	50.0	+0.391	0.5			Σ Cor = 11.3 mm
14	50.0	+0.291	0.4			Σ Cor = 11.7 mm
15	50.0	+0.111	0.1			Σ Cor = 11.7 mm
16	50.0	+0.274	0.3			Σ Cor = 11.8 mm
17	50.0	+0.288	0.4			Σ Cor = 11.9 mm
18	50.0	+0.314	0.4			Σ Cor = 12.0 mm
19	50.0	+0.330	0.4			Σ Cor = 12.1 mm
20	50.0	+0.062	0.1			
21	50.0	+0.334	0.4			Σ Cor = 12.3 mm
22	50.0	+0.745	0.9			
23	50.0	+2.792	3.4			Σ Cor = 20.0 mm
Bench DELFIN	50.0	+0.052	0.0			
Mark DELFIN		+2.184		10.000		

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ABSTRACT OF WYE LEVELING - ENGBI ZERO - Δ ELGIN TRAVERSE 12/16/47
Diff. in Elevation

FROM MARK	To Bench	From or B Pt.	Running Pt.	MEAN
ENGBI ZERO	ENGBI ZERO	+2.165	+2.118	+2.142
Bench ENGBI ZERO	1	+0.497	+0.373	+0.435
1	2	+0.498	+0.520	+0.509
2	3	+0.157	+0.247	+0.202
3	4	+0.725	+0.704	+0.715
4	5	+0.414	+0.398	+0.406
5	6	+0.127	+0.087	+0.107
6	7	+0.224	+0.227	+0.226
7	8	+0.429	+0.406	+0.418
8	9	+0.104	+0.145	+0.124
9	10	+0.175	+0.161	+0.168
10	11	+0.173	+0.176	+0.175
11	12	+0.267	+0.211	+0.239
12	13	+0.431	+0.441	+0.436
13	13+25	-0.157	+0.176	+0.014
13+25	13+25 Setup	-0.297	+0.343	+0.023
13+25 Setup	14	+0.283	+0.643	+0.463
14	15	+0.117	+0.111	+0.114
15	16	+0.27	+0.277	+0.274
16	17	+0.407	+0.389	+0.398
17	18	+0.27	+0.314	+0.292
18	19	+0.32	+0.335	+0.328
19	20	+0.032	+0.066	+0.049
20	21	+0.447	+0.428	+0.438
21	22	+0.734	+0.751	+0.743
22	23	+2.788	+2.808	+2.798
23	Bench Δ ELGIN	+0.051	+0.058	+0.055
Bench Δ ELGIN	MARK Δ ELGIN	+0.187	+0.184	+0.186
MARK Δ ELGIN	Water line at 1500 hrs	-0.581		-0.581

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MEAN TEMPERATURES FOR FULL TAPE LENGTHS (INVAR) - ENGBIZERO TOWER - DELGAIN.

~SBC~ STAKE Nos.	FWD. Thermo.	1/40 Average	
Bench Engob Zero - 1	28.1	28.1	28.1
1-2	28.5	28.5	28.5
2-3	29.3	29.3	29.3
3-4	29.5	29.5	29.5
4-5	29.2	29.2	29.2
5-6	29.5	29.5	29.5
6-7	29.6	29.6	29.6
7-8	29.5	29.5	29.5
8-9	29.5	29.5	29.5
9-10	29.3	29.3	29.3
10-11	29.7	29.7	29.7
11-12	29.1	29.1	29.1
12-13	30.1	30.1	30.1
13-14	29.7	29.7	29.7
14-15	29.0	29.0	29.0
15-16	29.3	29.3	29.3
16-17	29.1	29.1	29.1
17-18	32.0	32.0	32.0
18-19	30.3	30.3	30.3
19-20	29.5	29.5	29.5
20-21	29.8	29.8	29.8
21-22	30.1	30.1	30.1
22-23	30.7	30.7	30.7
23-Bench & DELGAIN	31.1	31.1	31.1

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COMPUTATIONS OF TESTED OFFSETS
AND OFFSETS FROM THE TIMING STATION
LENS FOR USE IN ESTABLISHING OTHER
POINTS

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COMPUTATION OF SET UPS AND SET BACKS FROM THE ZERO TOWER - DELGIN
(TURNING LINE) TRAVERSE TO PROPER DISTANCES FOR ESTABLISHING OTHER POINTS
BY CHORD OFFSETS

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OBJECT TO BE ESTABLISHED	DIST. FROM ZERO TOWER Feet	DIST. FROM TURNING LINE Feet	CLASS. OF CURVE	DIST. ZERO TOWER TO CLOSEST SET BACK	SET BACK FROM VERTICAL STA. Feet	SET BACK FROM VERTICAL STA. Feet
Hartman Sta.	1200'	365.74'	11'	1148.347'	1148.347'	1148.347'
Hartman Sta.	1500'	472.00'	11'	1454.607'	1454.607'	1454.607'
Hartman Sta.	1800'	578.26'	11'	1760.867'	1760.867'	1760.867'
Hartman Sta.	2100'	684.52'	11'	2067.127'	2067.127'	2067.127'
Hartman Sta.	2400'	790.78'	11'	2373.387'	2373.387'	2373.387'
Hartman Sta.	2700'	897.04'	11'	2679.647'	2679.647'	2679.647'
Hartman Sta.	3000'	1003.30'	11'	2985.907'	2985.907'	2985.907'
Hartman Sta.	3300'	1109.56'	11'	3292.167'	3292.167'	3292.167'
Hartman Sta.	3600'	1215.82'	11'	3598.427'	3598.427'	3598.427'
Hartman Sta.	3900'	1322.08'	11'	3904.687'	3904.687'	3904.687'

COMPUTATION OF SET UPS AND SET BACKS FROM THE ZERO TOWER - DELGIN
TRAVERSE TO PROPER DISTANCES FOR ESTABLISHING OTHER POINTS BY NORMAL OFFSETS

OBJECT TO BE ESTABLISHED	DISTANCE FROM ZERO TOWER	DISTANCE REQUIRED ON 2-T LINE	CLASS. OF CURVE	DIST. ZERO TOWER TO CLOSEST SET BACK	SET UP	SET BACK	LENGTH OF NORMAL OFFSET
Range Pole #1	1200'	1184.51'	11'	1148.347'	1148.347'	1148.347'	195.0'
GAMMA "A"	2250'	2249.95'	11'	2247.324'	2247.324'	2247.324'	14.71'
*Range Pole #2	—	2684.15'	11'	2746.423'	2746.423'	2746.423'	195.0'
*Range Pole #3	—	4184.11'	11'	4048.223'	4048.223'	4048.223'	195.0'

* Poles No. 2 + No. 3 were close to each other and the original range pole #3 fell on line between them to a large spot. The range pole on the other side of the zero tower. Pole #2 was on the other side of the zero tower. Pole #1 to original pole line. Pole #2 was on the other side of the zero tower. Pole #3 was on the other side of the zero tower. This is the line of poles from the range pole parallel to the zero tower turning line.

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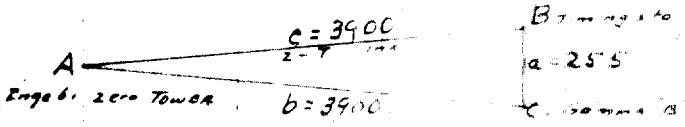
Engebi Island

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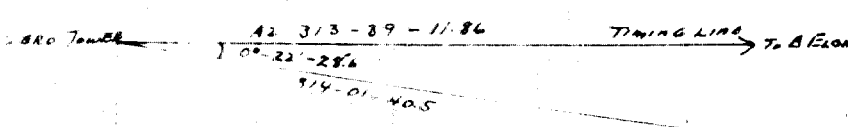
δ Zero-Time to γ B

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COMPUTATION OF CHORD AB to Chord offset from 1900 point on zero-time line to Gamma "B"

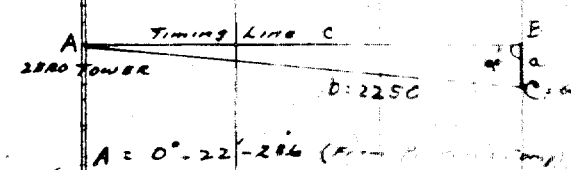


$a = 25.0^\circ$
 $b = 3900.0$
 $c = 3900.0$
 $2S = 7825.0$
 $S = 3912.5$
 $S-a = 3667.25$
 $S-b = 12.75$
 $S-c = 12.75$



$\log S-a = 3.589642$
 $\log S-b = 1.105510$
 $\log S-c = 1.105510$
 $\log S = 3.592482$
 $\log 2 = 1.040900$
 $\log \frac{1}{2}A = \frac{1}{5-a}$
 $\log 2 = 1.104090$
 $\log(S-a) = 3.589642$
 $\log \tan \frac{1}{2}A = 7.514447$
 $\frac{1}{2}A = 0^\circ 11' 14.3''$
 $A = 0^\circ 22' 28.6''$

COMPUTATION OF PROP. DIST. FROM TIMING LINE TO CHORD AB FROM NORMAL OFFSET FROM Z-T LINE TO GAMMA A AT 2250 FROM 2880 TOWER



$a = b \sin A$
 $\log 2250 = 3.3521825$
 $\log \sin A = 7.8184549$
 $\log a = 1.676374$
 $a = 14.71$

$c = b \cos A$
 $\log 2250 = 3.3521825$
 $\log \cos A = 9.9999915$
 $\log c = 3.3521740$
 $c = 2249.96$

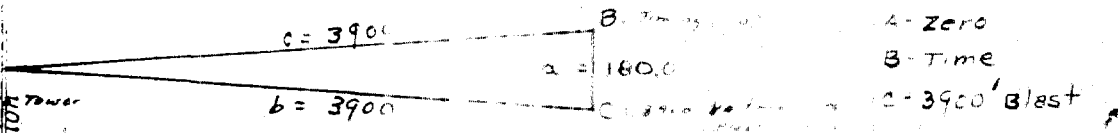
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Zero-Time Line to Blast Line

COMP. OF Chord AB for Chord Effects from ZERO Time to Hartman stations (Blast feeting)



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$a = 180'$
 $b = 3900'$
 $c = 3900'$
 $2s = 7980'$
 $s = 3990'$

$s-a = 3810'$
 $s-b = 90'$
 $s-c = 90'$

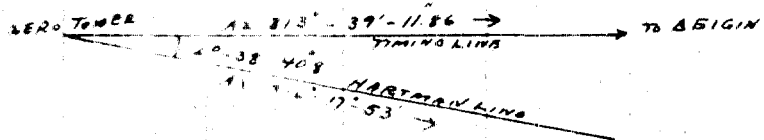
$\log(s-a) = 3.5809250'$
 $\log(s-b) = 1.9542425'$
 $\log(s-c) = 1.9542425'$
 $\log s = 3.6009729'$
 $\log r = 1.9442186'$

$\tan \frac{1}{2}A = \frac{r}{s-a}$

$\log r = 1.9442186'$
 $\log(s-a) = 3.5809250'$
 $\log \tan \frac{1}{2}A = 0.3632936'$
 $\frac{1}{2}A = 1^\circ 19' 20.4''$
 $A = 2^\circ 38' 40.8''$

check: $\cos A = \frac{b^2 + c^2 - a^2}{2bc}$

$b^2 = 15210000$
 $c^2 = 15210000$
 $b^2 + c^2 = 30420000 = 2bc$
 $a^2 = 32400$
 30387600



$\tan \frac{1}{2}B = \frac{r}{s-b} = \tan \frac{1}{2}C$

$\log r = 1.9442186'$
 $\log(s-b) = 1.9542425'$
 $\log \tan \frac{1}{2}B = 9.9899761'$
 $\frac{1}{2}B = 44^\circ 20' 19.8''$
 $B = 88^\circ 40' 39.6''$
 $C = 88^\circ 40' 39.6''$
 $r = 180.0000$

$\{ 30387600 = 7.4826964$
 $\{ 30420000 = 7.4831592$
 $\{ \cos A = 9.9995372$
 $2^\circ 38' 41''$

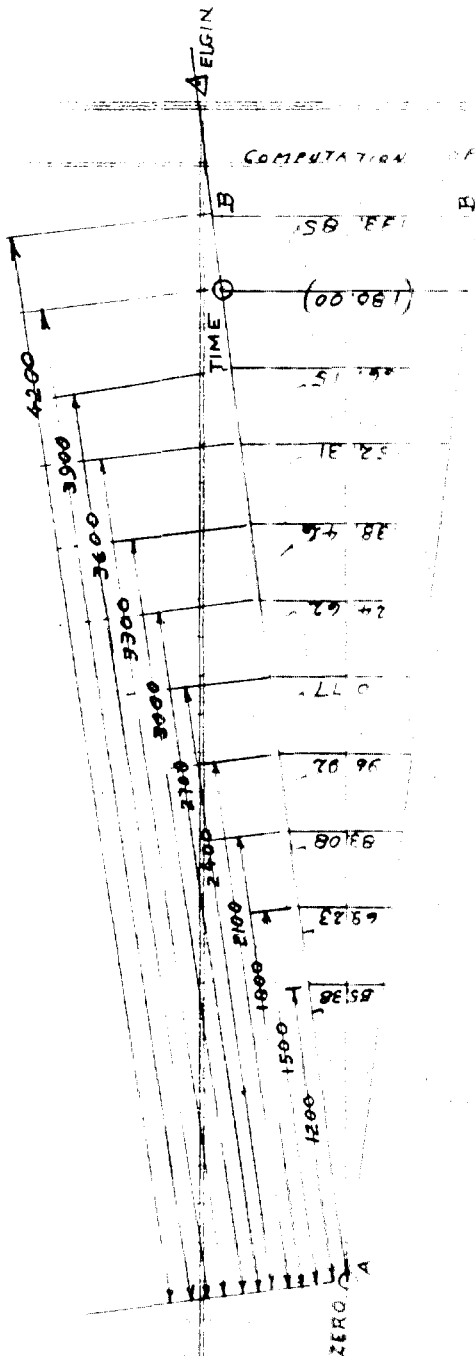
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100 yd. Blast Stations

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COMPUTATION OF CHORD DISTANCES to Hartman Stations From Z-T Line

2 A	38° 40' 8"	$a = \frac{b \sin A}{\sin B}$
2 B	40° 39' 6"	
log 200	2.301 030	log 2700 = 3.431 3638 ✓
log Sin A	8.664 094 ✓	log Sin A = 8.664 0941 ✓
	2.095 4579 ✓	
log Sin B	9.999 8843 ✓	log Sin B = 9.999 8843 ✓
	2.095 5736 ✓	
6. dist.	124.62 ✓	
log 500	2.698 970 ✓	log 3000 = 3.477 1213 ✓
log Sin A	8.664 094 ✓	log Sin A = 8.664 0941 ✓
	2.141 1854 ✓	
log Sin B	9.999 8843 ✓	log Sin B = 9.999 8843 ✓
	2.141 3311 ✓	
7. dist.	138.46 ✓	
log 600	2.778 153 ✓	log 3300 = 3.518 5139 ✓
log Sin A	8.664 094 ✓	log Sin A = 8.664 0941 ✓
	2.182 3666 ✓	
log Sin B	9.999 8843 ✓	log Sin B = 9.999 8843 ✓
	2.182 7237 ✓	
8. dist.	152.31 ✓	
log 800	2.903 090 ✓	log 3600 = 3.556 3025 ✓
log Sin A	8.664 094 ✓	log Sin A = 8.664 0941 ✓
	2.220 3134 ✓	
log Sin B	9.999 8843 ✓	log Sin B = 9.999 8843 ✓
	2.220 5123 ✓	
9. dist.	166.15 ✓	
log 1000	3.000 000 ✓	log 4200 = 3.623 2493 ✓
log Sin A	8.664 094 ✓	log Sin A = 8.664 0941 ✓
	2.287 3053 ✓	
log Sin B	9.999 8843 ✓	log Sin B = 9.999 8843 ✓
	2.287 4591 ✓	
10. dist.	193.85 ✓	

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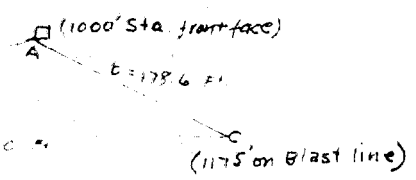
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Form 665
Ed. Dec. 1929

TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$\frac{a}{b} = \tan(45^\circ + \phi)$ (Calc longer side a) $\tan \frac{1}{2}(A_D - B_D) = \tan \phi \tan \frac{1}{2}(A_D + B_D)$; $c = \frac{a \sin C_D}{\sin A_D}$ *
 Comp of Dist. from Zero Tower To Near face of 1000 ionization station. 71

C_D		Log c	3.070 0379	Log m	
$\frac{\text{Sph. excess}}{3}$		Log b	0.584 177	Log $\sin C_D$	
C_D	10° 31' 36"	Log $\tan B$	0.245 524	Log a	
$90^\circ - \frac{1}{2}C_D = \frac{1}{2}(A_D + B_D)$	5° 15' 48"	$(45^\circ + \phi)$	0.26	Log b	
$(A_D - B_D)$	8° 16' 24"	ϕ	0.26	Log sph. ex.	
$\text{Sum} = A_D$	16° 32' 48"	Log $\tan \frac{1}{2}(A_D - B_D)$	0.664 9438	Sph. excess	--
$\text{Diff} = B_D$	8° 16' 24"	Log $\tan \frac{1}{2}(A_D + B_D)$	0.732 0379		
	10° 31' 36"		0.897 0284		
			(Sketch)		
Log a	3.070 0379				
Log $\sin C_D$	9.26 177				
Colog $\sin A_D$	0.666 85				
Log c	3.070 0379	(Zero)			



CHECK COMPUTATION

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.000 0147
1	0-39-40				0.732 8291
2	1-53-36				8.519 0254
3	167-26-44				9.237 941
1-3				117.595	2.251 8692
1-2				1175.00	3.070 0379
	180 00 00				
2-3					
1					
2					
3					
1-3					
1-2					

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*The subscripts a, b, c on this form refer to spherical and plane angles respectively.

77A

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Pole #1 from Z-T Line

Comp of Dist out from Tower along Z-T Line for Normal offset to Range pole #1 (1200' from 2)

B Range pole #1

c = 1200

a = 105

A =

ZERO TANG

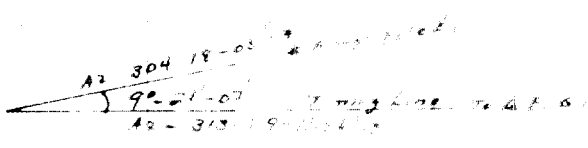
$\log 145 = 2.159 0346$
 $\log 1200 = 3.079 1812$
 $\log \sin 9.210 = 2.534$
 $\log 97.210 = 1.988$

b = c Cos A

$\log 1200 = 3.079 1812$
 $\log \cos 9.210 = 9.994 2097$
 $\log b = 3.073 3909$

b = 124.11 Dist out from Z-T Line for Normal offset to pole #1
 1500.00 spacing between Poles (Pole line 11 to Z-T Line)
 26.84 11 Dist out from Z-T Line for Normal offset to pole #2 *
 1500.00
 4.84 11 Dist out from Z-T Line for Normal offset to pole #3 *

* Pole #3 later moved - pole #3 moved Northward
 perpendicular to ^{original} line of poles
 Pole #3 moved Northward 200 on line L to original
 line of poles - Thus poles are still on a straight
 line, but line is no longer B to Z-T Line.
 (See next sheet for Range Poles
 #1 & #2 as re-set.



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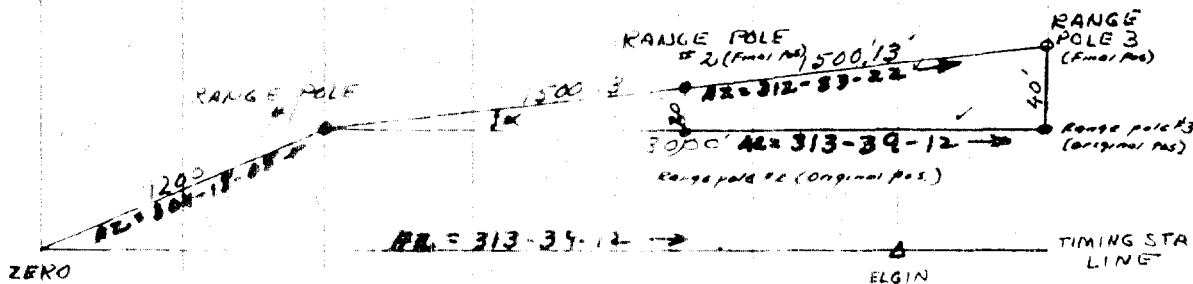
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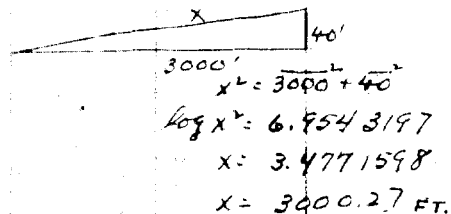
RANGE POLES ENGEBI

Distances + Azimuth - Range Poles, attn. Poles #2 + #3 were re-set. (copy)

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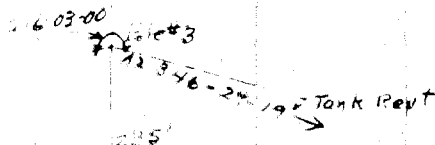
log 40	1.6020600
1000	3.4771213
log tan α	2.1453277
α	11° 42' 50"



Azimuth of Range Pole #1 313° 39' 12"



log 30	1.4771213
4184	3.6215911
log tan β	2.7494762
β	10° 15'



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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$$\left[\frac{a}{b} = \tan(45^\circ \pm \phi) \pm \tan \phi \tan \frac{1}{2}(A_p + B_p); \quad c = \frac{a \sin C_p}{\sin A_p} \right]^*$$

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C_p	100° 00' 00"	100.0000	100.0000	2.412	Log m
$\frac{\text{Sph. excess}}{3}$				12.00	Log sin C_p
C_p	100° 00' 00"	100.0000	100.0000	12.12	Log a
$\frac{1}{2} C_p$	50° 00' 00"	50.0000	50.0000	12.70	Log b
$90^\circ - \frac{1}{2} C_p = \frac{1}{2}(A_p - B_p)$	40° 00' 00"	40.0000	40.0000	12.70	Log sph. ex.
$\frac{1}{2}(A_p - B_p)$	40° 00' 00"	40.0000	40.0000	12.361	Sph. excess
Sum = A_p	100° 00' 00"	100.0000	100.0000	8.182	
Diff = B_p	100° 00' 00"	100.0000	100.0000	8.543	
C_p				(Sketch)	

Log a	2.0000	2.412
Log sin C_p	9.896	12.12
Colog sin A_p	0.094	12.00
Log c	3.672	3.918

CHECK COMPUTATION

No.	STATION	SIDE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.672 3918
1					0.103 4679
2					8.428 2637
3	A				9.905 3815
1-3				160 0	2.204 1234
1-2				4800 0	3.681 2412

2-3					
1					
2					
3	2800 To B.C.				
1-3					
1-2					

*The subscript s and p of this form refer to spherical and plane respectively.

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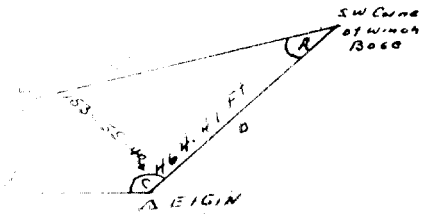
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Form 665
Ed. Dec. 1929

TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

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$$\left[\frac{a}{b} = \tan(45^\circ + \phi) \quad (\text{Call longer side } a) \quad \text{and } \frac{a}{b} = \tan \frac{1}{2}(A_p + B_p) \quad c = \frac{a \sin C_p}{\sin A_p} \right]^*$$

C_s		Log a	3.608 3924	Log m	
$\frac{\text{Sph. excess}}{3}$		Log b	2.666 8982	Log $\sin C_s$	
C_p	153 5 00	Log $\tan \frac{1}{2}(A_p + B_p)$	4.607 4607	Log a	
$\frac{1}{2} C_p$	76 17 00	$(A_p + B_p)$	23 24 21.6	Log b	
$90^\circ - \frac{1}{2} C_p = \frac{1}{2}(A_p + B_p)$	13 47 11.6	$\frac{1}{2}(A_p + B_p)$	11 52 10.8	Log sph. ex.	
$\frac{1}{2}(A_p - B_p)$	10 24 00.0	Log $\tan \frac{1}{2}(A_p - B_p)$	2.192 1973	Sph. excess	
Sum = A_p	23 39 00.0	Log $\tan \frac{1}{2}(A_p + B_p) \pm \frac{1}{2}(A_p - B_p)$	3.404 6113		
Diff = B_p	12 11 29.3	Log $\tan \frac{1}{2}(A_p + B_p) \mp \frac{1}{2}(A_p - B_p)$	2.414 7186		
C_p	153 55 40.0		(St. Job)		
Log a	3.608 3924				
Log $\sin C_p$	9.642 3356				
Colog $\sin A_p$	0.399 9748				
Log c	3.651 3315				



4480.0 CHECK COMPUTATION

No.	STATION	SPHERICAL EXCESS	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.651 3315
1					0.357 0374
2					9.600 6235
3					8.658 5293
1-3					3.608 3924
1-2					2.666 8982
2-3					
1					
2					
3					
1-3					
1-2					

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*The subscript s and p of the form refer to spherical and plane angle respectively.

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COMPUTATIONS AND MEASUREMENTS
REQUIRED FOR LOCATING AND
CHECKING THE Bu Y&D STRUCTURES

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12-23-47

ENGINEER I

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ENGINEER I 2- Bu Y4D to chord on edge Runway.

COMPUTATION For Chord offset stake on N side of runway

	North	
	c = 1050	r = 50
	b = 1050	
a:	50	
b:	1050	
c:	1050	
2S:	2150	
S:	1075	
S-a:	1025	
S-b:	25	
S-c:	25	
log (s-a):	3.010 7239	
log (s-b):	1.397 9400	
log (s-c):	1.397 9400	
	5.806 6039	
log s:	3.031 4085	
	2.775 1954	
log r:	1.387 5977	
log r =	1.387 5977	
log (s-a):	3.010 7239	
log tan 1/2 A:	8.376 8735	
1/2 A:	10 21' 27"	
A:	20 43' 42"	
log r =	1.387 5977	
log (s-b):	1.397 9400	
log tan 1/2 B:	9.989 6577	
1/2 B:	44 19' 04"	
B:	88 38' 08"	

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ENGINEER I

12-23-47

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ENGINEER I 2-Bo y & L S. to locate on edge of Runway.
COMPUTATION FOR CHORD offset stakes on all 4 sides of runway.

SOLUTION		CHECK	
C=1500		C=1500	
a =	288	a =	148
b =	1500	b =	1600
c =	1500	c =	1500
2s =	3288	2s =	3148
s =	1644	s =	1574
s-a =	1356	s-a =	1426
s-b =	144	s-b =	74
s-c =	144	s-c =	74
log(s-a) =	3.132 2597	log(s-a) =	3.154 1195
log(s-b) =	2.158 3625	log(s-b) =	1.869 2317
log(s-c) =	2.158 3625	log(s-c) =	1.869 2317
	7.448 9847		6.892 5829
log s =	3.215 9018	log s =	3.197 0047
	4.233 0819		3.695 5782
log r =	2.116 5415	log r =	1.847 7891
log r =	2.116 5415	log r =	1.847 7891
log(s-a) =	3.132 2597	log(s-a) =	3.154 1195
log tan 1/2 A =	8.984 2818	log tan 1/2 B =	8.693 2696
1/2 A =	5° 30' 00"	1/2 B =	49° 39.8'
A =	11° 01' 00"	B =	99° 79.6'
log r =	2.116 5415	log r =	1.847 7891
log(s-b) =	2.158 3625	log(s-b) =	1.869 2317
log tan 1/2 B =	9.958 1791	log tan 1/2 A =	8.978 5574
1/2 B =	42° 14' 44"	1/2 A =	43° 35' 10"
B =	84° 29' 28"	A =	87° 10' 20"

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COMPUTATION FOR CHORD AND STAKES ON SIDES OF RUNWAY

SOUTH		NORTH	
c = 2100		c = 2100	
L = 2100		L = 2100	
a = 224		a = 224	
b = 2100		b = 2100	
c = 2100		c = 2100	
2S = 4424		2S = 4424	
S = 2212		S = 2212	
S-a = 1968		S-a = 1988	
S-b = 132		S-b = 114	
S-c = 132		S-c = 112	
log(S-a) 3.294 0.51		log(S-a) 3.298 4.64	
log(S-b) 2.120 5.39		log(S-b) 2.049 2.10	
log(S-c) 2.20 5.35		log(S-c) 2.049 2.10	
7.535 1.29		7.396 8.524	
log S 3.348 6.42		log S 3.344 7.851	
4.18 4.00		4.052 0.673	
log r 2.093 2.37		log r 2.026 0.336	
log 2 = 2.093 2.374		log 2 = 2.026 0.336	
log(S-a) 3.294 0.51		log(S-a) 3.298 4.64	
log tan 1/2 A 8.771 2.14		log tan 1/2 A 8.727 6.172	
1/2 A 3° 36' 38"		1/2 A 2° 03' 26.0"	
A 7° 12' 17.7"		A 4° 06' 52.0"	
log r 2.093 2.374		log r 2.026 0.336	
log(S-b) 2.120 5.39		log(S-b) 2.049 2.10	
log tan 1/2 B 9.976 6.655		log tan 1/2 B 9.976 8.156	
1/2 B 43° 11' 52.0"		1/2 B 43° 28' 17.0"	
B 86° 23' 44.0"		B 86° 56' 34.0"	

B
a = 224

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TARGET Z: En. Y & Z to Chords on edge Runway
COMPUTATION FOR CHAIN OF SET STAKES ON N+S SIDES OF RUNWAY

SOUTH

NORTH

$c = 2550$
 $b = 2550$
 $a = 272$
 $b = 2550$
 $c = 2550$
 $s = 5322$
 $s = 2661$
 $s-a = 2439$
 $s-b =$
 $s-c =$
 $\log(s-a) = 3.387\ 2118$
 $\log(s-b) = 2.045\ 3230$
 $\log(s-c) = 2.045\ 3230$
 $\frac{7.477\ 8578}{\log s = 3.425\ 0449}$
 $4.052\ 8129$
 $\log r = 2.026\ 4065$
 $\log r = 2.026\ 4065$
 $\log(s-a) = 3.387\ 2118$
 $\log \tan \frac{1}{2} A = 8.639\ 1947$
 $\frac{1}{2} A = 20^\circ\ 29'\ 41''$
 $A = 40^\circ\ 59'\ 82''$
 $\log r = 2.026\ 4065$
 $\log(s-b) = 2.045\ 3230$
 $\log \tan \frac{1}{2} B = 9.981\ 0877$
 $\frac{1}{2} B = 43^\circ\ 45'\ 09''$
 $B = 87^\circ\ 30'\ 18''$

2 222

$c = 2550$
 $a = 242$
 $b = 2550$
 $a = 242$
 $b = 2550$
 $c = 2550$
 $s = 5342$
 $s = 2671$
 $s-a = 2429$
 $s-b = 121$
 $s-c = 121$
 $\log(s-a) = 3.385\ 4275$
 $\log(s-b) = 2.082\ 7854$
 $\log(s-c) = 2.082\ 7854$
 $\frac{7.550\ 9983}{\log s = 3.426\ 6739}$
 $4.124\ 3244$
 $\log r = 2.062\ 1622$
 $\log r = 2.062\ 1622$
 $\log(s-a) = 3.385\ 4275$
 $\log \tan \frac{1}{2} A = 8.676\ 7347$
 $\frac{1}{2} A = 20^\circ\ 43'\ 11''$
 $A = 40^\circ\ 26'\ 22''$
 $\log r = 2.062\ 1622$
 $\log(s-b) = 2.082\ 7854$
 $\log \tan \frac{1}{2} B = 9.979\ 3768$
 $\frac{1}{2} B = 43^\circ\ 38'\ 24''$
 $B = 87^\circ\ 16'\ 48''$

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COMP for Chord offset stakes on sides of Runway

SOUTH

$c = 3000$
 $b = 3000$

$a = 246$
 $b = 3000$
 $c = 3000$

$2S = 62146$
 $S = 31073$

$S-a = 2871$
 $S-b = 1237$
 $S-c = 1237$

$\log(S-a) = 3.458 \ 9314$
 $\log(S-b) = 2.089 \ 9016$
 $\log(S-c) = 2.089 \ 9016$

$\log S = 3.494 \ 5710$
 $4.145 \ 1721$

$\log R = 2.072 \ 2830$

$\tan \frac{1}{2}A = \frac{r}{S-a}$

$\log R = 2.072 \ 0810$
 $\log(S-a) = 3.458 \ 9314$
 $\log \tan \frac{1}{2}A = 8.613 \ 1451$

$\frac{1}{2}A = 2^\circ - 25' - 53.2$
 $A = 4^\circ - 41' - 50.4$

$\tan \frac{1}{2}B = \frac{r}{S-b} = \tan \frac{1}{2}C$

$\log R = 2.072 \ 0810$
 $\log S-b = 2.089 \ 9016$
 $\log \tan \frac{1}{2}B = 9.982 \ 1897$

$\frac{1}{2}B = 43^\circ - 49' - 30.07$
 $87^\circ - 39' - 00.14$

NORTH

$c = 3000$
 $b = 3000$

$a = 284$
 $b = 3000$
 $c = 3000$

$2S = 62146$
 $S = 31073$

$S-a = 2871$
 $S-b = 1237$
 $S-c = 1237$

$\log(S-a) = 3.456 \ 0622$
 $\log(S-b) = 2.092 \ 2883$
 $\log(S-c) = 2.092 \ 2883$

$\log S = 3.497 \ 2062$
 $4.263 \ 4326$

$\log R = 2.131 \ 7163$

$\tan \frac{1}{2}A = \frac{r}{S-a}$

$\log R = 2.131 \ 7163$
 $\log(S-a) = 3.456 \ 0622$
 $\log \tan \frac{1}{2}A = 8.675 \ 6541$

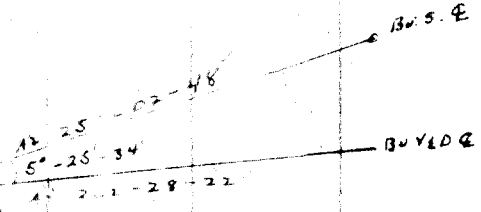
$\frac{1}{2}A = 2^\circ - 21' - 43.468$
 $A = 4^\circ - 43' - 23.6$

$\tan \frac{1}{2}B = \frac{r}{S-b} = \tan \frac{1}{2}C$

$\log R = 2.131 \ 7163$
 $\log(S-b) = 2.092 \ 2883$
 $\log \tan \frac{1}{2}B = 9.977 \ 4280$

$\frac{1}{2}B = 43^\circ - 48' - 38.366$
 $87^\circ - 37' - 13.2$

* This Chord Stake Later used as
3000' & Stake of Bu.S. Line.



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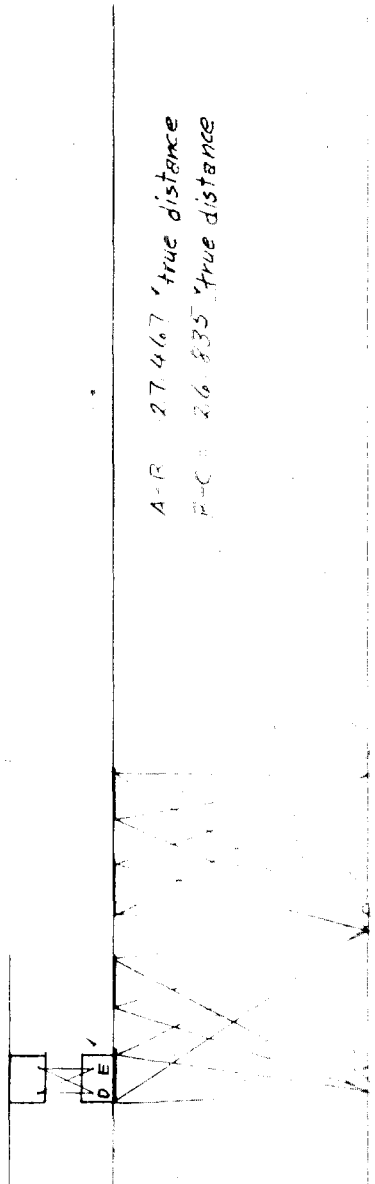
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~~SECRET~~
U.S.C. & G.S. Tape #3774

NOTE - ALL MEASUREMENTS ARE
from the END of the tape and NOT
from the ZERO POINT for true distance.
ADD 0.45 FT. to the following

1050' CUBES

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A-R 27.467' true distance
P-C 26.835' true distance

Feet	
1	27.467
2	27.467
3	27.467
4	27.467
5	27.467
6	27.467
7	27.467
8	27.467
9	27.467
10	27.467
11	27.467
12	27.467
13	27.467
14	27.467
15	27.467
16	27.467
17	27.467
18	27.467
19	27.467
20	27.467
21	27.467
22	27.467
23	27.467
24	27.467
25	27.467
26	27.467
27	27.467
28	27.467
29	27.467
30	27.467
31	27.467
32	27.467
33	27.467
34	27.467
35	27.467
36	27.467
37	27.467
38	27.467
39	27.467
40	27.467
41	27.467
42	27.467
43	27.467
44	27.467
45	27.467
46	27.467
47	27.467
48	27.467
49	27.467
50	27.467
51	27.467
52	27.467
53	27.467
54	27.467
55	27.467
56	27.467
57	27.467
58	27.467
59	27.467
60	27.467
61	27.467
62	27.467
63	27.467
64	27.467
65	27.467
66	27.467
67	27.467
68	27.467
69	27.467
70	27.467
71	27.467
72	27.467
73	27.467
74	27.467
75	27.467
76	27.467
77	27.467
78	27.467
79	27.467
80	27.467
81	27.467
82	27.467
83	27.467
84	27.467
85	27.467
86	27.467
87	27.467
88	27.467
89	27.467
90	27.467
91	27.467
92	27.467
93	27.467
94	27.467
95	27.467
96	27.467
97	27.467
98	27.467
99	27.467
100	27.467

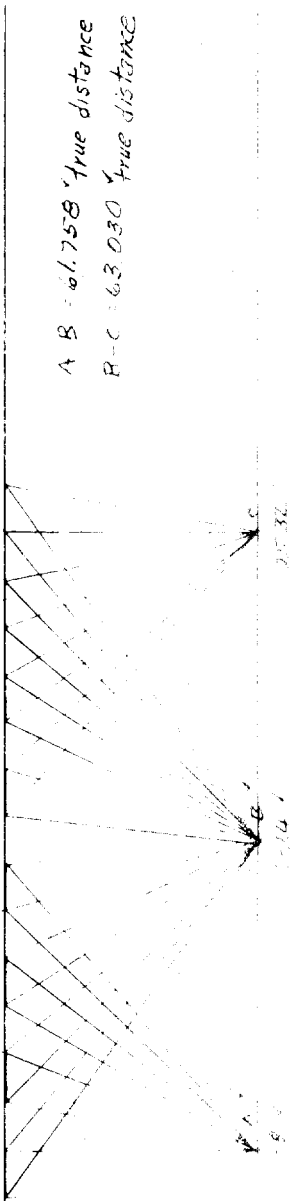
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~~SECRET~~
U.S. G.S. Tape # 3714

NOTE - This information
from the ...
from the ...
ADD 0.4 ...

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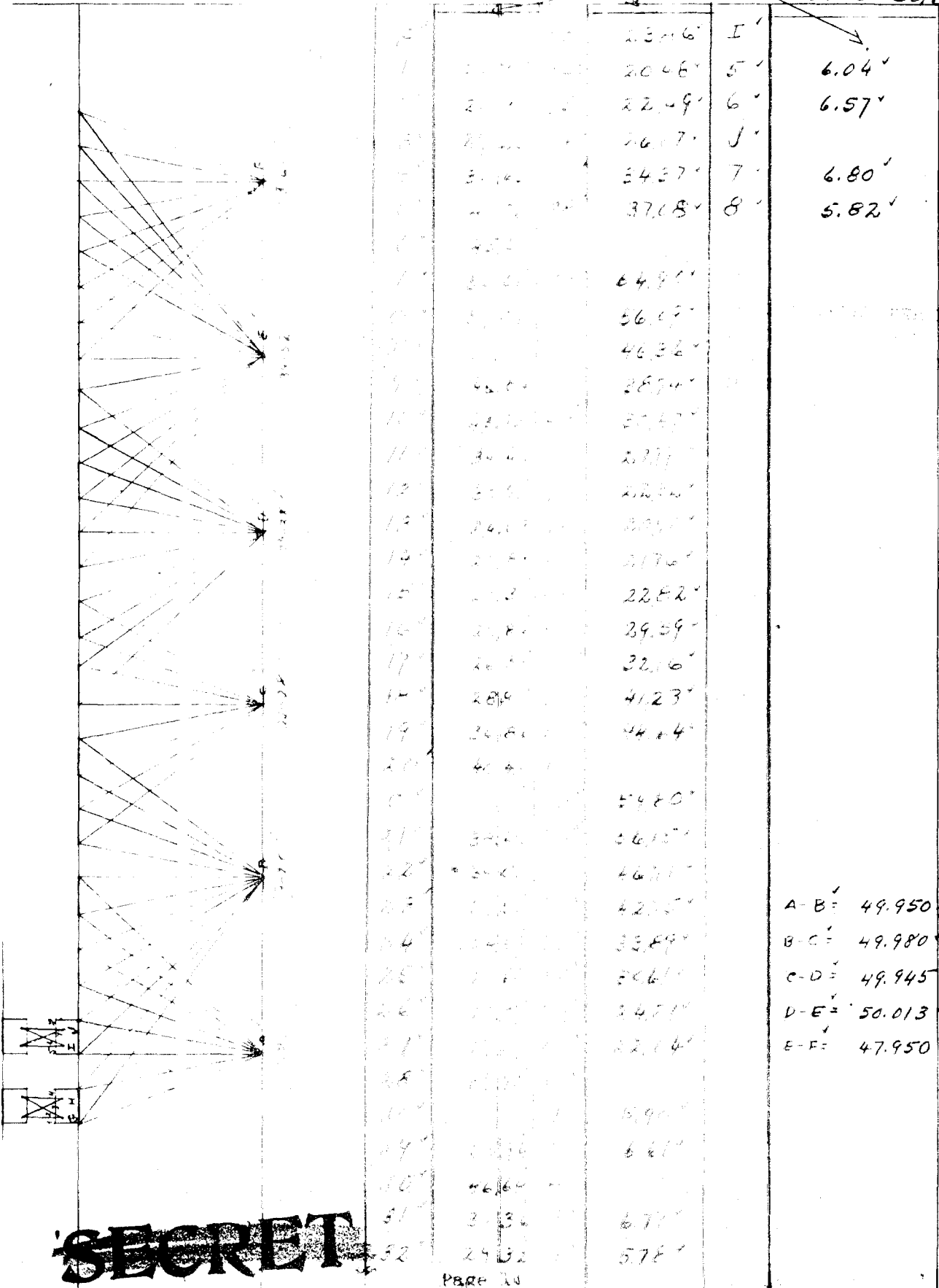
1	10.00
2	10.00
3	10.00
4	10.00
5	10.00
6	10.00
7	10.00
8	10.00
9	10.00
10	10.00
11	10.00
12	10.00
13	10.00
14	10.00
15	10.00
16	10.00
17	10.00
18	10.00
19	10.00
20	10.00
21	10.00
22	10.00
23	10.00
24	10.00
25	10.00
26	10.00
27	10.00
28	10.00
29	10.00
30	10.00
31	10.00
32	10.00
33	10.00
34	10.00
35	10.00
36	10.00
37	10.00
38	10.00
39	10.00
40	10.00
41	10.00
42	10.00
43	10.00
44	10.00
45	10.00
46	10.00
47	10.00
48	10.00
49	10.00
50	10.00
51	10.00
52	10.00
53	10.00
54	10.00
55	10.00
56	10.00
57	10.00
58	10.00
59	10.00
60	10.00
61	10.00
62	10.00
63	10.00
64	10.00
65	10.00
66	10.00
67	10.00
68	10.00
69	10.00
70	10.00
71	10.00
72	10.00
73	10.00
74	10.00
75	10.00
76	10.00
77	10.00
78	10.00
79	10.00
80	10.00
81	10.00
82	10.00
83	10.00
84	10.00
85	10.00
86	10.00
87	10.00
88	10.00
89	10.00
90	10.00
91	10.00
92	10.00
93	10.00
94	10.00
95	10.00
96	10.00
97	10.00
98	10.00
99	10.00
100	10.00

1500 CUBES

~~SECRET~~

NOTE - These measurements
from the EN...
from the ...
ADD 0.415 ...

2,100' CUBES



A-B = 49.950' true dist
 B-C = 49.980' " "
 C-D = 49.945' " "
 D-E = 50.013' " "
 E-F = 47.950' " "

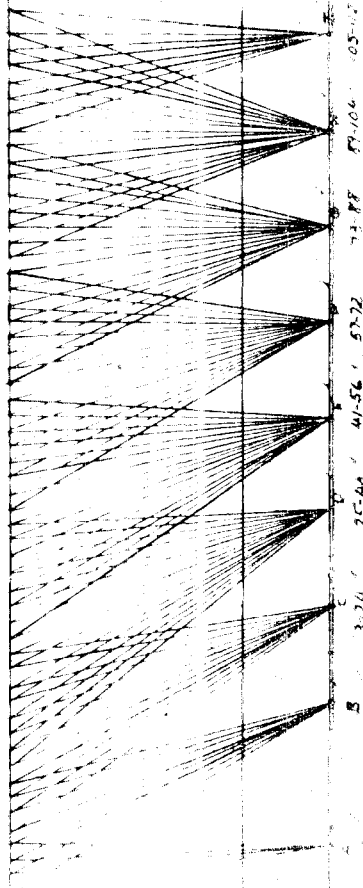
SECRET

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2550 CUBES

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2550 RADIAL
LINE OF BU YAD
CUBES

A-B	50.030' true dist.
B-C	50.118' "
C-D	49.775' "
D-E	49.933' "
E-F	49.990' "
F-G	49.840' "
G-H	50.145' "
H-I	50.042' "

USX #65 Tape #3774

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NOTE - These measurements are from the END of the tape and NOT from the ZERO end for the ...
 ADD 34 ...

~~SECRET~~

2550' CUBES

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A		33			21.80	4505
1	19.65	34	3.14	68	22.38	5035
2	20.65	35	2.85	69	29.85	60.34
3	23.58	36	2.72	70	31.90	67.5
4	25.72	37	2.55	7	40.0	
B		38	2.00	72	49.95	4567
5	56.11	39	1.95	6		4099
6	53.24	40	1.80	7	60.40	30.31
7	41.57	41	1.70	72	67.0	28.47
8	39.31	42	1.60	73	77.5	22.75
9	29.30	43	1.50	74	84.2	19.98
10	26.92	44	1.40	75	91.0	18.33
11	20.80	45	1.30	76	98.0	12.46
12	19.99	46	1.20	80	104.0	
C		47	1.10	81	110.0	
13	74.62	48	1.00	82	116.0	
14	71.25	49	0.90	83	122.0	
15	60.84	50	0.80	84	128.0	
16	57.56	51	0.70	85	134.0	
17	47.57	52	0.60	86	140.0	
18	44.53	53	0.50	87	146.0	
19	35.21	54	0.40	88	152.0	
20	32.51	55	0.30	89	158.0	
21	24.61	56	0.20	90	164.0	
22	22.50	57	0.10	91	170.0	
23	19.72	F		92	176.0	
24	20.40	58	0.00	93	182.0	
D		59		94	188.0	
25	95.12	60		95	194.0	
26	91.75	61		96	200.0	
27	81.30	62		97	206.0	
28	77.97	63		98	212.0	
29	67.42	64		99	218.0	
30	63.39	65		100	224.0	
31	53.19	66		101	230.0	
32	49.42			102	236.0	

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USCGS TAPE
 # 3774

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NOTE - These measurements are taken from the END of the tape and not from the ZERO point for time of flight. ADD 0.4 to the following

U.S.C.&G.S. Tape #3774

3000' CUBES

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SAME AS SKETCH FOR 2100' CUBES

A-B	50,000'
B-C	49,930'
C-D	49,990'
D-E	50,100'
E-F	49,920'

4'		2524'	I'	5.91'
5'	24'	2299'	5'	6.71'
6'	48'	2075'	6'	6.69'
7'	72'	1851'	7'	5.99'
8'	96'	1627'	8'	
9'	120'	1403'		
10'	144'	1179'		
11'	168'	955'		
12'	192'	731'		
13'	216'	507'		
14'	240'	283'		
15'	264'	59'		
16'	288'	115'		
17'	312'	291'		
18'	336'	467'		
19'	360'	643'		
20'	384'	819'		
21'	408'	995'		
22'	432'	1171'		
23'	456'	1347'		
24'	480'	1523'		
25'	504'	1699'		
26'	528'	1875'		
27'	552'	2051'		
28'	576'	2227'		
29'	600'	2403'		
30'	624'	2579'		
31'	648'	2755'		
32'	672'	2931'		
33'	696'	3107'		
34'	720'	3283'		
35'	744'	3459'		
36'	768'	3635'		
37'	792'	3811'		
38'	816'	3987'		
39'	840'	4163'		
40'	864'	4339'		
41'	888'	4515'		
42'	912'	4691'		
43'	936'	4867'		
44'	960'	5043'		
45'	984'	5219'		
46'	1008'	5395'		
47'	1032'	5571'		
48'	1056'	5747'		
49'	1080'	5923'		
50'	1104'	6099'		
51'	1128'	6275'		
52'	1152'	6451'		
53'	1176'	6627'		
54'	1200'	6803'		
55'	1224'	6979'		
56'	1248'	7155'		
57'	1272'	7331'		
58'	1296'	7507'		
59'	1320'	7683'		
60'	1344'	7859'		
61'	1368'	8035'		
62'	1392'	8211'		
63'	1416'	8387'		
64'	1440'	8563'		
65'	1464'	8739'		
66'	1488'	8915'		
67'	1512'	9091'		
68'	1536'	9267'		
69'	1560'	9443'		
70'	1584'	9619'		
71'	1608'	9795'		
72'	1632'	9971'		
73'	1656'	10147'		
74'	1680'	10323'		
75'	1704'	10499'		
76'	1728'	10675'		
77'	1752'	10851'		
78'	1776'	11027'		
79'	1800'	11203'		
80'	1824'	11379'		
81'	1848'	11555'		
82'	1872'	11731'		
83'	1896'	11907'		
84'	1920'	12083'		
85'	1944'	12259'		
86'	1968'	12435'		
87'	1992'	12611'		
88'	2016'	12787'		
89'	2040'	12963'		
90'	2064'	13139'		
91'	2088'	13315'		
92'	2112'	13491'		
93'	2136'	13667'		
94'	2160'	13843'		
95'	2184'	14019'		
96'	2208'	14195'		
97'	2232'	14371'		
98'	2256'	14547'		
99'	2280'	14723'		
100'	2304'	14899'		

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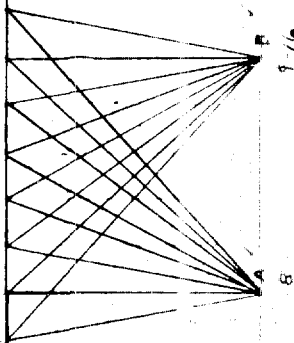
~~SECRET~~

NOTE - These measurements are taken
from the END of the tape and NOT
from the zero mark. For true distances
ADD 0.4 to the following

U.S.C.&G.S. Tape # 3774

~~OFFICIAL USE ONLY~~

3600' CUBES



A-B is 9.16 true dist.

1	21.96'
2	19.04'
3	20.10'
4	23.32'
5	30.58'
6	35.77'
7	48.10'
8	52.95'
9	
10	63.10'
11	57.69'
12	47.19'
13	42.12'
14	32.27'
15	28.32'
16	20.98'
17	20.64'

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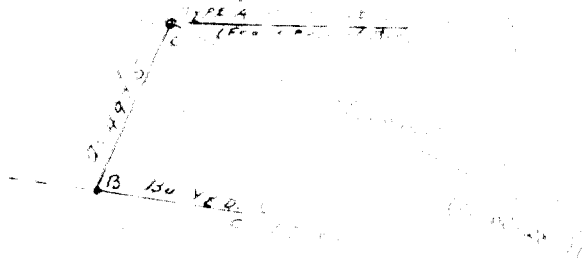
CONCLUSIONS AND RECOMMENDATIONS
RESPECTIVE FOR LOGGERS
OF THE S. BILLYN S.

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COMPUTATION OF S.F. ... ON BUYED & TO OCE RING TYPE A
(1500' FROM ZERO)



$a = 1500$ $(1500) = 1500$
 $b = 1500$ $(1500) = 1500$
 $c = 1500$ $(1500) = 1500$
 $2s = 3497.5$
 $s = 1748.75$

$\text{Log}(s-a) = 3.097348$
 $\text{Log}(s-b) = 2.897348$
 $\text{Log}(s-c) = 2.395148$
 $2 = 2.888576$
 $\text{Log } s = 3.242727$
 4.646744
 $\text{Log } A = 2.823011$
 $\text{Log}(s-a) = 3.097348$
 $\text{Tan } \frac{1}{2} A = 9.287117$
 $\frac{1}{2} A = 9.143558$
 $A = 18.287117$

$\text{Log}(s-a) = 3.097348$
 $\text{Log}(s-b) = 2.897348$
 $\text{Log}(s-c) = 2.395148$
 $2 = 2.888576$
 $\text{Log } s = 3.242727$
 4.646744
 $\text{Log } A = 2.823011$
 $\text{Log}(s-a) = 3.097348$
 $\text{Tan } \frac{1}{2} A = 9.287117$
 $\frac{1}{2} A = 9.143558$
 $A = 18.287117$

SECRET

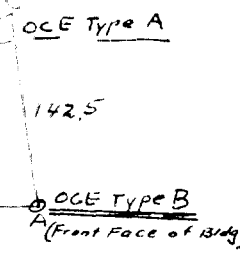
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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$$\left[\frac{a}{b} = \tan(45^\circ + \phi) \quad \text{Call long. side } m \quad \tan \phi = \frac{m - b}{m + b} \quad \tan \phi \tan \frac{1}{2}(A_p + B_p) \quad c = \frac{a \sin C_p}{\sin A_p} \right]^*$$

COMP. OF Dist from ZERO + A from OCE B. LOG A for OCE B. LOG ¹⁹²⁸ STAKE SET BY 18th Engrs. 41
(Stake originally set by USCGS (1901) & checked out by USCGS (1928) 1874 Engrs)

C_s	81 11 00.0	Log m	1.912
$\frac{\text{Sph. excess}}{3}$		Log sin C_s	8.49
C_p	81 11 00.0	Log a	2.64
$\frac{1}{2} C_p$	40 27 30.0	Log b	2.35
$90^\circ - \frac{1}{2} C_p = \frac{1}{2}(A_p + B_p)$	48 21 30.0	Log sph. ex.	2.35
$\frac{1}{2}(A_p - B_p)$	40 27 30.0	Sph. excess	2345
Sum = A_p	87 11 17.0		5997
Diff = B_p	05 26 46.8		8342
C_p	81 11 00.0	(Sketch)	
Log a	3.016 0915		
Log sin C_p	9.489 6116		
Colog sin A_p	0.160 4976		
Log c	3.146 0915		
	1.500 0000	CHECK COMPUTATION	



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No.	STATION	DESCRIPTION	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					
1					
2					
3					
1-3					
1-2					
2-3					
1					
2					
3					
1-3					
1-2					

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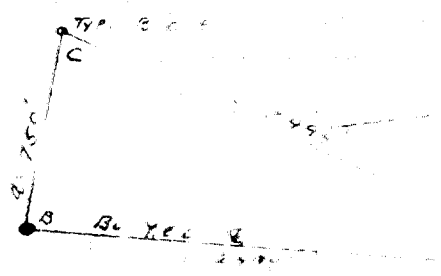
*The subscripts 1, 2, 3, are taken from the sides of the triangle and angle respectively.

1480 100
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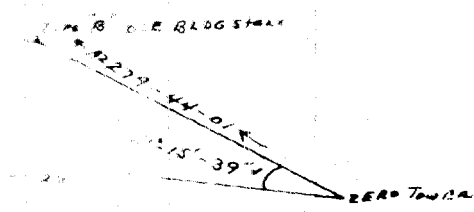
COMPUTATION OF ΔB for Coord off set from 2494 point on BU YED Q To O.C.E BLDG. TYPE B (2499' from Z)



NOTE: IN Building this structure
Bette. boards were set so
that Front Face of structure
is 2500' From ZERO TOWER

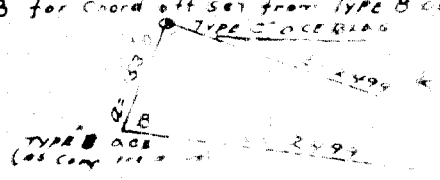
a = 750.0
b = 2499
c = 2499
2S = 5748
S = 2874

Log (s-a) = 3.327 1540	Log 2 = 2.508 3652
Log (s-b) = 2.574 0310	Log (s-b) = 2.574 0313
Log (s-c) = 2.574 0310	Log tan 1/2 B = 9.934 3339
Σ 8.475 217	1/2 C = 40 41 05.3
Log S = 3.458 4868	B = 81 22 10.6
5.016 7303	C = 81 22 10.6
Log 2 = 2.508 3652	A = 17 15 39.0
	Σ 180 00 00.2



COMPUTATION OF ΔB for Coord off set from TYPE B O.C.E. BLDG. (2499' from Z) To TYPE 'C' O.C.E. BLDG. (2499' from Z)

a = 153
b = 2499
c = 2499



NOTE: IN Building to this stake,
Bldg. was erected so as to have
a dist of 2500' to back of
4' Crown at top

2S = 5151 (s-a) = 2422.5
S = 2575.5 (s-b) = 76.5
(s-c) = 76.5

Log (s-a) = 3.384 2638	Log 2 = 1.870 3625
Log (s-b) = 1.883 6614	Log (s-b) = 1.883 6614
Log (s-c) = 1.883 6614	Log tan 1/2 B = 9.986 7011
Σ 7.151 5866	1/2 C = 44-07-22.4
Log S = 3.410 8616	B = 48 14 44.8
3.740 7250	C = 48 14 44.8
Log 2 = 1.870 3625	A = 3 30 30.4
	Σ 180 00 00.0

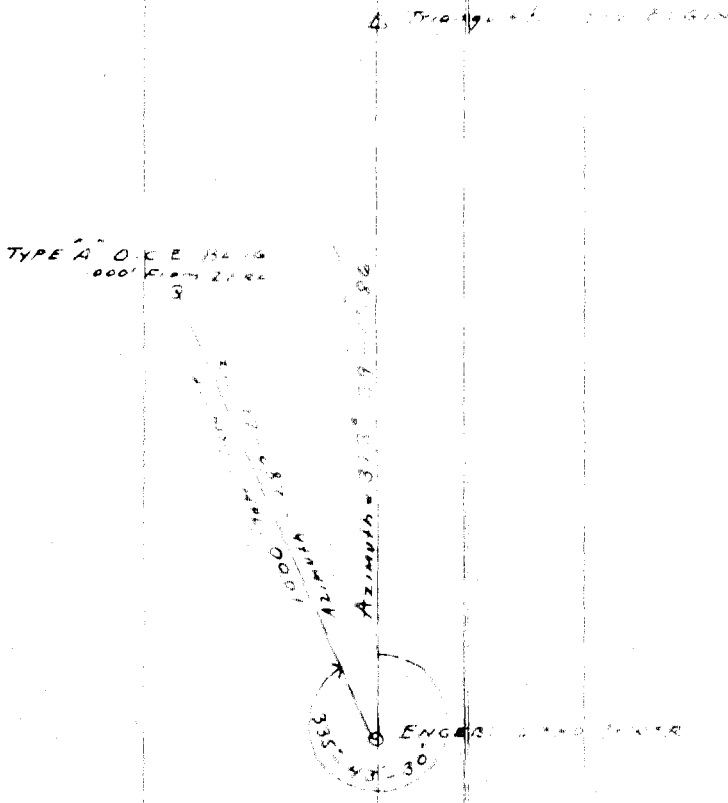
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SKETCH SHOWING LOCATION OF TYPE A OKE BEING (1000 FEET FROM ZERO TOWER)



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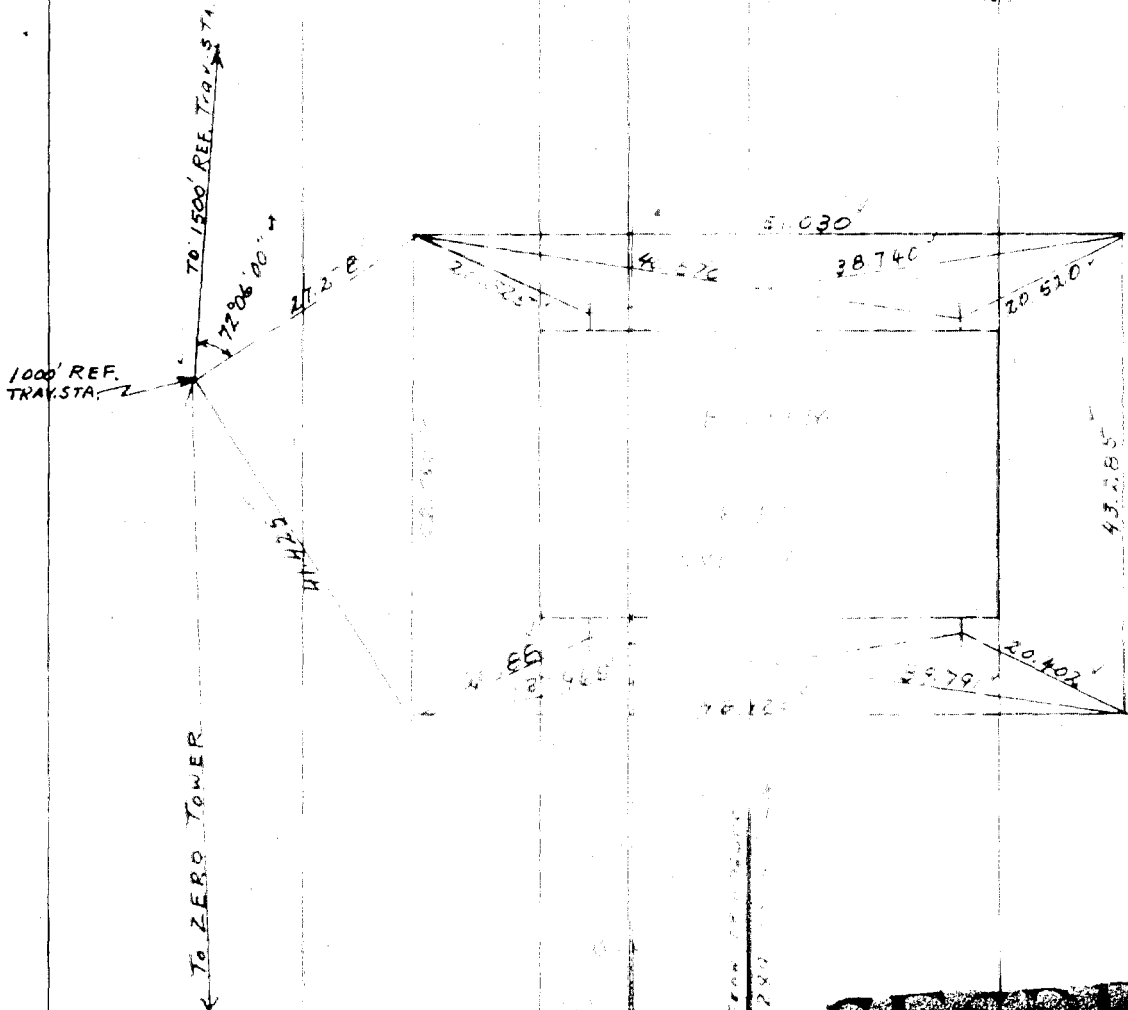
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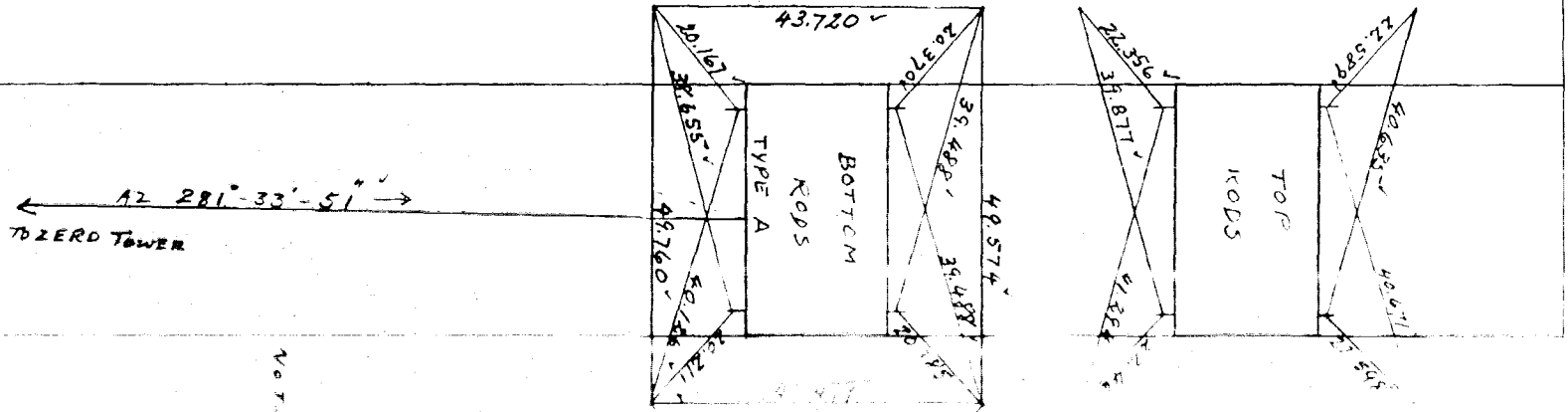
1000 O.C.F. BLDG.

DECLASSIFIED BY: [unclear]
ON: [unclear]
REASON: [unclear]
DATE: [unclear]

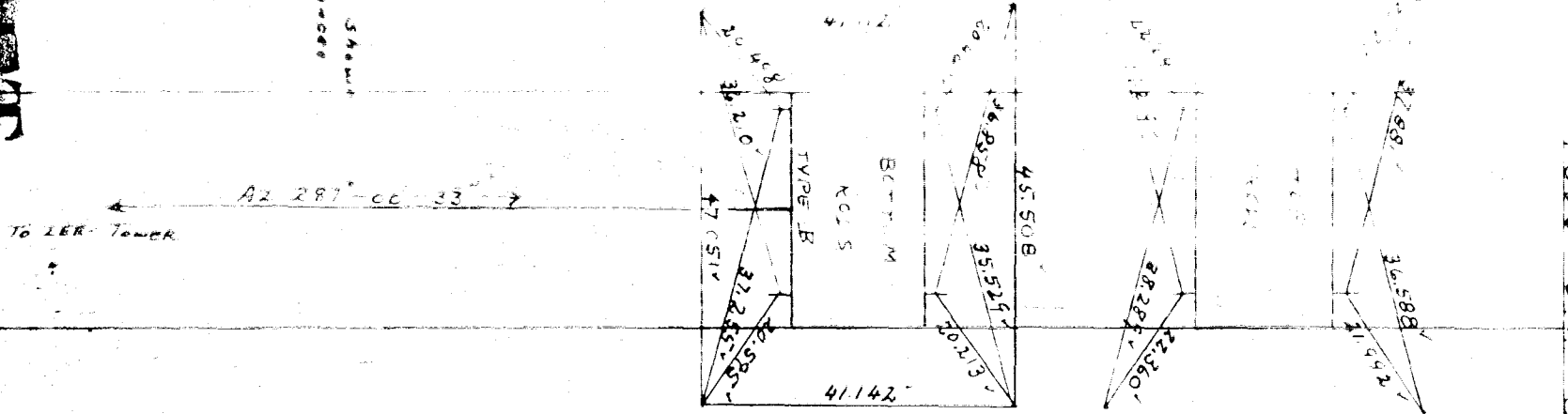
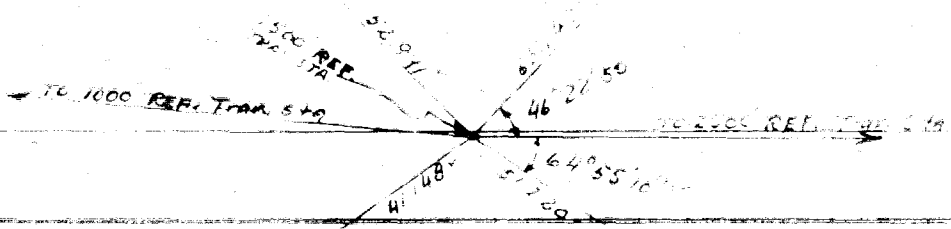
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NOTE: All distances shown
are slope distances.





NOTE: ALL DISTANCES SHOWN
ON STOPS & SPANCS



1500 OCE BLDGS.

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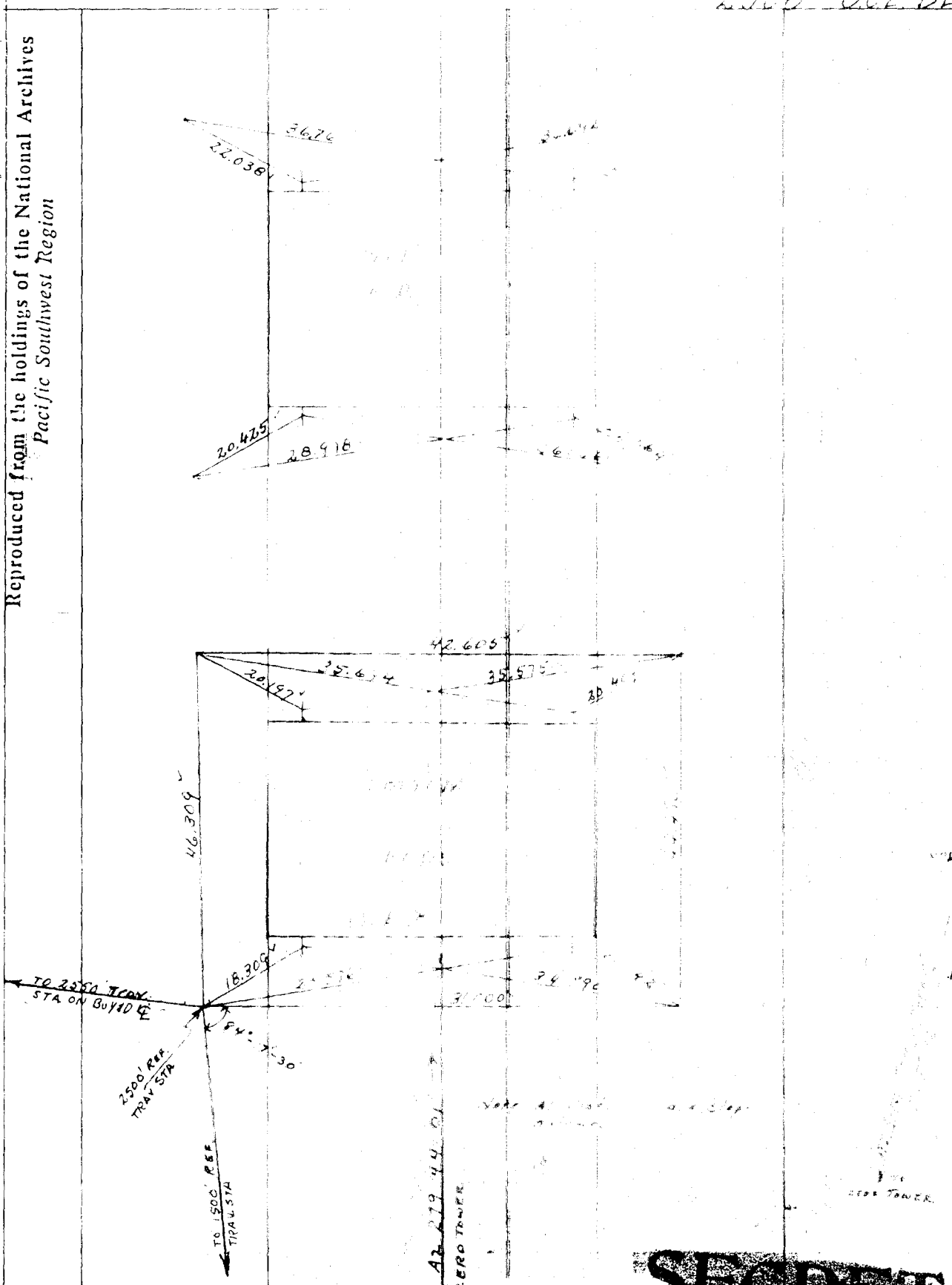
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2500 OCE BLDG

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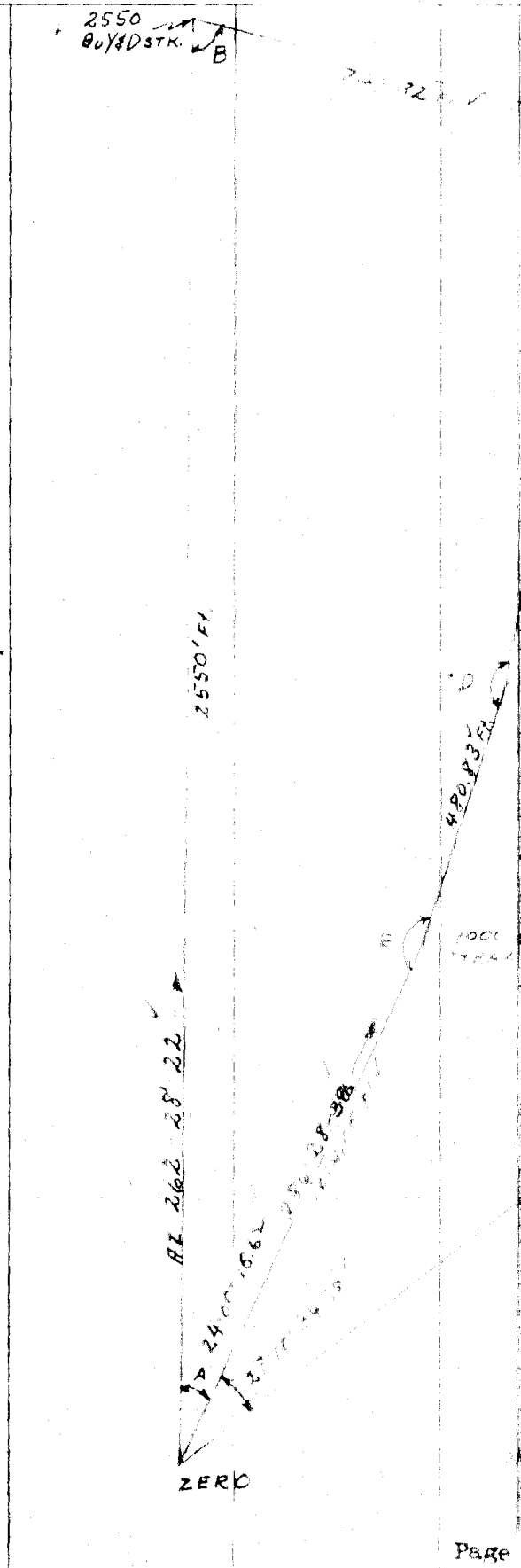
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C.C.E. TRAVERSE



Observed Δ		Corrected Δ			
		Cor.			
24	00	15.62	-1.2'	24° 00'	14.4'
70	17	15.84	-1.2'	76° 17'	14.6'
94	19	04.69	-1.2'	94° 19'	03.5'
177	16	39.69	-1.2'	171° 16'	38.5'
174	06	50.19	-1.2'	174° 06'	49.0'
24	00	06.03		540 00	00.6'

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BUS LINE

3000 STK
 2900 STK
 2800 STK
 2700 STK
 2600 STK
 2500 STK
 2400 STK
 2300 STK
 2250 STK

3000' BUSY STK

A.P.C. Camps RZ. 257 02 46

ZERO

The 3000' E stake of the
 ... the 3000' chord stake
 ... the N side of the runway
 ... the BUSY Line. (Computation
 ... BUSY Section of Engeri Camps)
 ... this 3000' chord stake,
 ... rods were set at
 ... intervals on line to
 ... as indicated

... other Bus Stop point
 ... the 4200' blast

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1947 11-17-47

V C

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VAR 200
EPK 200

100-100000-100-100000

100

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ACRON - BELLID - ROJCA

Location of Structures from the Zero Tower

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Pacific Southwest Region

STRUCTURE	DISTANCE FROM ZERO	AZIMUTH FROM ZERO
Gamma Station A	2130 ft	006 17 38
" " B	3900 "	" " "
" " C	5400 "	" " "
Blast Footings	1200 "	004 05 24
" "	1500 "	" " "
" "	1800 "	" " "
" "	2100 "	" " "
" "	2400	" " "
" "	2700 "	" " "
" "	3000 "	" " "
" "	3300 "	" " "
" "	3600 "	" " "
" "	3900 "	" " "
" "	4200 "	" " "
" "	4450 "	" " "
Blast Building	4300 "	001 39 27
Timing Station	5900 "	001 53 10
Center one of line of Bu Y&D conc. cubes	1050 "	275 08 07 *
" "	1500 "	285 53 41 *
" "	2000 "	287 39 13 *
" "	3000 "	297 05 13 292° 40' 45"
" "	3600 "	294 20 08 *
" "	4050 "	296 14 31 *
Ion. Station	1000 "	002 13 24
Range Pole #1	1500 "	004 25 57
" " #2	3840 "	007 43 02
" " #3	4190 "	008 53 19
Triang. Sta. Graflex	558.415 "	001 40 30
Photo Tower	630.435 "	005 30 52
Tank Revetment	4278 ft	003 30 32
Winch Base	4240 "	003 17 12

* Since preparing this Table of Distances from the Zero Tower, it has been reported that all of these stakes have been removed or destroyed and that location of Bu Y&D units will be determined by resection from other points.

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GEOGRAPHIC POSITIONS

STRUCTURE	LATITUDE	LONGITUDE
Zero Tower	11 37 26.884	162 19 11.614 ✓
Photo Tower	11 37 31.984	162 19 15.277 ✓
Traverse Station Aomon	11 37 11.884	162 19 27.578 ✓
Traverse Station Bjiiri	11 37 01.084	162 19 47.779 ✓
Triangulation Station Graflex	11 37 21.884	162 19 14.568 ✓

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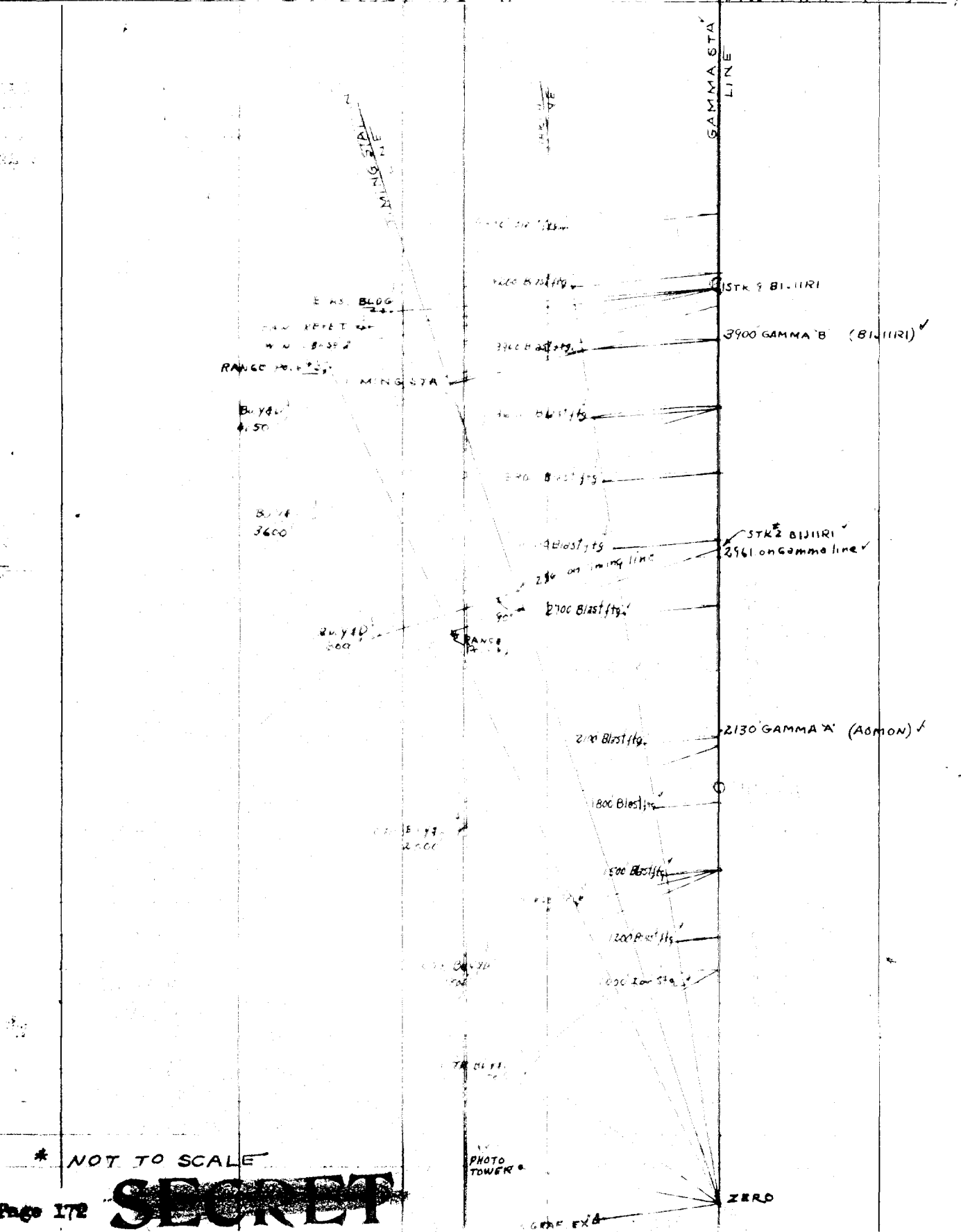
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ADMON: EILKIKI...

5400 GAMMA 'C' (NOJA) ✓

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Pacific Southwest Region



* NOT TO SCALE

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NOJA

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COMPUTATIONS OF SETUPS, SETBACKS
AND OFFSETS FROM THE GAMMA STATION
LINE FOR USE IN ESTABLISHING OTHER
POINTS

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COMPUTATION OF "SET UPS" AND "SET BACKS" FROM STAKES OF THE GAMMA LINE TRAVERSE TO PROPER DISTANCES FOR FOR ESTABLISHMENT OF OTHER POINTS BY CHORD OFFSETS

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OBJECT TO BE ESTABLISHED	DIST. FROM ZERO TOWER FEET	DIST. FROM ZERO TOWER METERS	CLOSEST STAKE TOWER TO CLOSEST STAKE METERS	SET UP From Closest Stk Ft.	SET BACK From Closest Stk Ft.
Ion STA	1000	304.80	299.46	5.34	
Bu. YED	1050	320.16	299.46	20.70	
Hartman Sta.	1200	365.82	299.46	66.36	
Hartman Sta. Range Fw 81	1500	457.20	299.46	157.74	
Hartman Sta.	1800	548.58	299.46	249.12	132.43
Bu. YED	2050	624.90	299.46	325.44	4.509
Hartman Sta.	2100	640.26	299.46	340.80	
GAMMA A	2130	649.80	299.46	350.34	2.550
Hartman Sta.	2200	670.56	299.46	371.10	
Hartman Sta.	2000	609.60	299.46	310.14	
Hartman Sta.	3300	1005.84	299.46	706.38	3.3985
Hartman Sta.	2600	792.48	299.46	493.02	11.150
Bu. S	3750	1143.00	299.46	843.54	39.216
TINING STA GAMMA B Hartman Sta.	3900	1188.60	299.46	889.14	
HARTMAN STA	4200	1280.40	299.46	980.94	67.382
Bu. YED	4050	1234.50	299.46	935.04	81.415
Hartman Sta.	4500	1371.00	299.46	1071.54	
GAMMA C	5400	1643.16	299.46	1343.70	

* This stake found high part of island and the distance of line to zero tower is 1000 feet.

copy 0-20
10-21

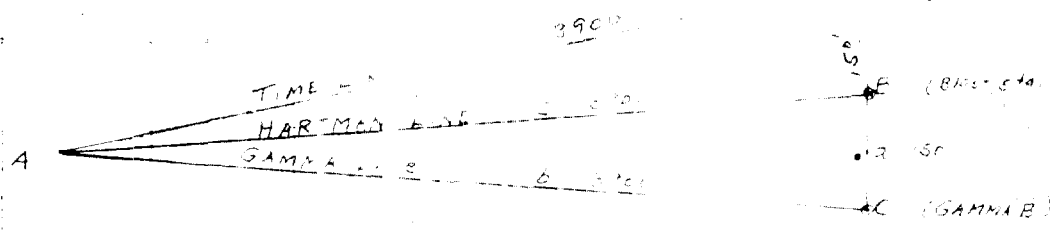
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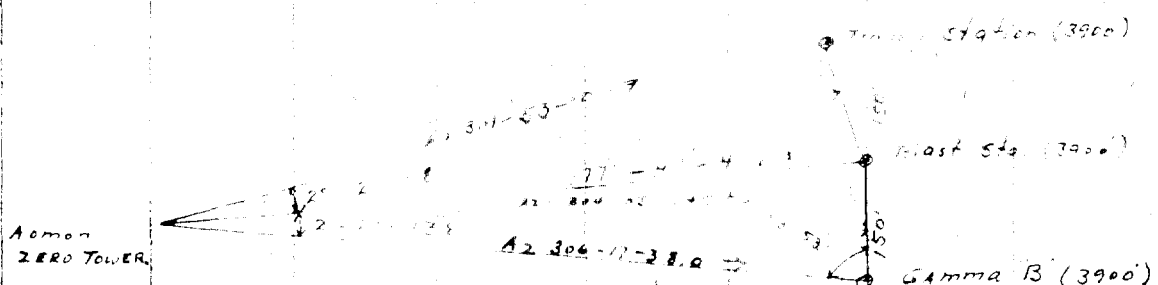
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Pacific Southwest Region

COMPUTATION OF Δ FOR 150' Chord offset from 3900' Point on GAMMA LINE To 3900' Hartman Sta. Timing station



$a = 150$
 $b = 3900$
 $c = 3900$
 $2s = 7950$
 $s = 3975$
 $s-a = 3825$
 $s-b = 75$
 $s-c = 75$

$\log(s-a)$	8.582	$\log a$	1.866	$\log 2$	1.866
$\log(s-b)$	1.875	$\log(s-c)$	1.875	$\log(s-b)$	1.875
$\log(s-c)$	1.875	$\log \tan \frac{1}{2} B$	9.991	$\log \tan \frac{1}{2} B$	9.991
Σ	7.332			$\frac{1}{2} B$	44° 26' 56.55"
$\log s$	3.595			B	88° 53' 53.1"
	3.733			C	88° 53' 53.1"
$\log h$	1.866			A	2° 12' 13.81"
					180° 00' 00.0"



Timing station stake set by 150' chord offset from 3900' Hartman Sta. stake as shown in above sketch.

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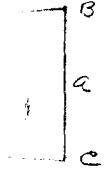
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CHORD DISTANCE GAMMA LINE TO Hartman Line

A

A = 20° - 12' - 17.8"
B = 88° - 53' - 53"
C = 88° - 53' - 53"
180 - C = 91.466

SABIAN LINE



Log 1200 3.079 182
Log Sin A 8.584 9491
Σ 1.664 131
Log Sin B 9.999 9197
Log Dist 1.664 248
Dist 46.54

Log 1500 3.176 091
Log Sin A 8.924 9197
Σ 1.748 008
Log Sin B 9.999 9197
Log Dist 1.748 131
Dist 62.43

Log 1800 3.255 2725
Log Sin A 8.584 9491
Σ 1.840 2216
Log Sin B 9.999 9197
Log Dist 1.840 3019
Dist 69.231 ✓

Log 2100 3.322 2
Log Sin A 8.584 9491
Σ 1.907 165
Log Sin B 9.999 9197
Log Dist 1.907 248
Dist 80.770

Log 2700 3.431 3678
Log Sin A 8.784 9491
Σ 2.212 319
Log Sin B 9.999 9197
Log Dist 2.212 399
Dist 123.24

Log 3000 3.477 1213 *
Log Sin A 8.584 9491 ✓
Σ 2.062 0704
Log Sin B 9.999 9197
Log Dist 2.062 1507
Dist 115.385 ✓

Log 3300 3.518 57
Log Sin A 8.584 9491
Σ 2.103 46
Log Sin B 9.999 9197
Log Dist 2.103 54
Dist 126.92

Log 3600 3.559 92
Log Sin A 8.849 9491
Σ 2.244 2
Log Sin B 9.999 9197
Log Dist 2.244 10
Dist 138.76

Log 4500 * 3.653 2125 ✓
Log Sin A 8.584 9491
Σ 2.238 1616
Log Sin B 9.999 9197
Log Dist 2.238 2419
Dist 173.078 ✓

(Distance computation...)

3900' Blast started at 150 chord section
Gamma Line)

Log 4200 3.623 2493
Log Sin A 8.584 9491
Σ 2.208 1984
Log Sin B 9.999 9197
Log Dist 2.208 2787
Dist 161.540

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98A

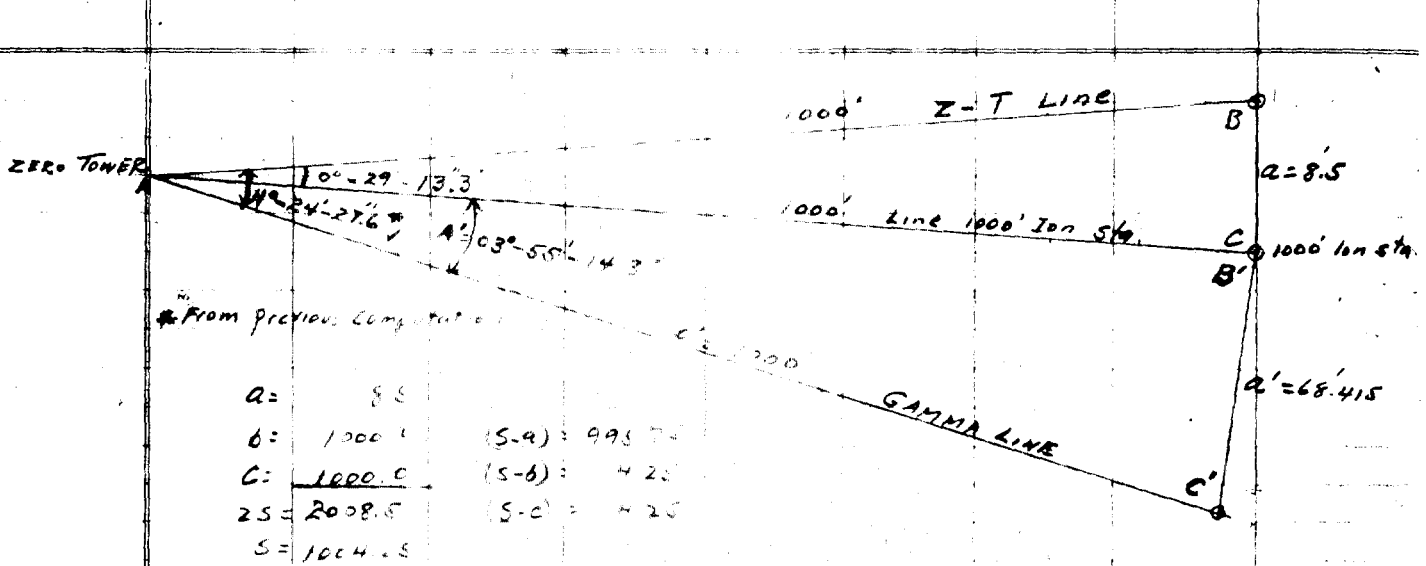
* This stake was set back 50.0 Towards Zero Tower by Island Comdr.
Giving a distance from 200 Tower of 4450 Feet.

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Pacific Southwest Region

Comp. of chord dist. 25' for chord offset from GAMMA LINE to 1000' Ionization Sta



*From previous computation

$a = 8.5$
 $b = 1000$ $(s-a) = 998.75$
 $c = 1000.0$ $(s-b) = 425$
 $2s = 2008.5$ $(s-c) = 425$
 $s = 1004.5$

$\text{Log}(s-a)$	2.998 1503	$\text{Log } r =$	0.626 5432	$\text{Log } r =$	0.626 5432
$\text{Log}(s-b)$	0.628 3889	$\text{Log}(s-a)$	2.998 1503	$\text{Log}(s-b)$	0.628 3889
$\text{Log}(s-c)$	0.628 3889	$\text{Log tan } \frac{1}{2} A$	7.628 3929	$\text{Log tan } \frac{1}{2} B$	9.998 1543
Σ	4.254 9281	$\frac{1}{2} A =$	0°-14'-36.63	$\frac{1}{2} B =$	44°-52'-41.72
$\text{Log } s =$	3.001 8418	$A =$	0°-29'-13.3	$B =$	89°-45'-23.4
	1.253 0863			$C =$	89°-45'-23.4
$\text{Log } r =$	0.626 5432			$A =$	0°-29'-13.3
					180 00 00.1

$A' = (03-55-14.3)$
 $B' = (88-02-22.8)$
 $C' = (88-02-22.8)$

$a' = \frac{b \sin A'}{\sin B}$

$\text{Log } 1000 = 3.000 0000$
 $\text{Log } \sin A = 8.834 8952$
 $\Sigma = 1.834 8952$
 $\text{Log } \sin B = 9.999 7458$
 $\text{Log } a' \text{ Dist} = 1.835 1494$
 $a = 68.415 \text{ Ft}$

= Chord dist from 1000' point on GAMMA LINE
To center of front of 1000' Ion Station
Nearest Zero Tower.

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Comp of Chord Δ s + OFFSETS for location of Range pole #1

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ZERO TOWER A

TIME LINE

GAMMA LINE

$A = 40-24-27.0$ ← 1000 DIST

$B = (87-47-46.2)$

$C = (87-47-46.2)$

$a = \frac{b \sin A}{\sin B}$

Log 1500 =	3.176 093	Log 2850 =	3.454 802	Log 4200 =	3.623 2493
Log sin A	8.885 6577	Log sin A	8.885 6577	Log sin A	8.885 6577
Σ	2.061 7490	Σ	2.340 5016	Σ	2.508 9070
Log sin B	9.999 6786	Log sin B	9.999 6786	Log sin B	9.999 6786
Log Dist	2.062 0704	Log Dist	2.340 8224	Log Dist	2.509 2284
DIST	115.364	DIST	219.92	DIST	323.019

(checked by S. J. ...)

Range pole #1

Range pole #2

Range pole #3

ZERO TOWER A

TIME LINE

$a = \frac{195}{1500}$ (c-a) 1402.5

$b = \frac{1500}{1500}$ (c-b) 92.5

$c = \frac{1500}{1500}$ (c-c) 0

$2S = 3195$

$S = 1597.5$

Log (S-a) =	3.146 9025	Log (S-b) =	1.989 0046	Log (S-c) =	1.989 0046
Σ	2.124 912	Log S =	3.203 440	Log 10 =	1.960 7356
Log S =	3.203 440	Log 10 =	1.960 7356	Log (S-b)	1.989 0046
Log 10 =	1.960 7356	Log 10 =	1.960 7356	Log 10 =	1.960 7356

Range pole #2

Range pole #3

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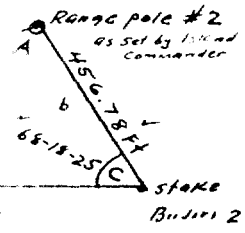
TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$\frac{a}{b} = \tan(45^\circ + \phi)$ (Call angle ϕ the angle between the sides a and b) $\tan \phi = \frac{a - b}{a + b}$ $\phi = \tan^{-1} \frac{a - b}{a + b}$ $c = \frac{a \sin C_p}{\sin A_p}$ *

(A)

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Pacific Southwest Region

C_p		Log	4.74 6504	Log m	
$\frac{\text{Sph. excess}}{3}$		Log	2.659 7071	Log $\sin C_p$	
C_p	68-18-25	Log \sin	9.944 9433	Log a	
$\frac{1}{2} C_p$	34-09-12.5	Log \cos	9.915 1839	Log b	
$90^\circ - \frac{1}{2} C_p = \frac{1}{2}(A_p + B_p)$	55-50-47.5	Log \sin	9.917 0839	Log sph. ex.	
$\frac{1}{2}(A_p - B_p)$	10-06-58.8	Log \cos	9.965 9396	Sph. excess	—
Sum = A_p	103 06 58.8	Log \sin	9.968 5060		
Diff = B_p	08 34 36.2	Log \cos	9.984 4456		
C_p	68 18 25		(Sketch)		
	180 00 00.0				
Log a	3.474 6504				
Log $\sin C_p$	9.968 5060				
Colog $\sin A_p$	0.031 9014				
Log c	3.454 2296				



CHECK COMPUTATION

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.454 2296
1	C			68-18-25	0.031 9014
2	B			08-34-36.2	9.173 5756
3	A			103-06-58.8	9.988 5194
1-3				56.78	2.659 7066
1-2				2982.98	3.474 6504
2-3					
1					
2					
3					
1-3					
1-2					

*The subscripts s and p refer to spherical and plane angles respectively

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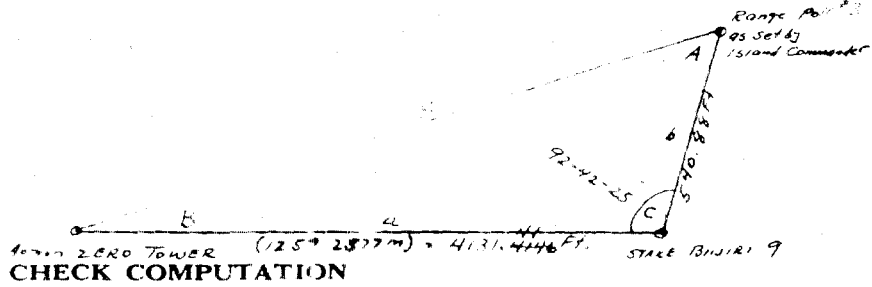
TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

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$$\left[\frac{a}{b} = \tan(45^\circ + \phi) \right] \text{ (Call longer side } a, \text{ shorter side } b, \text{ and } \phi = \frac{1}{2}(A_p + B_p); \quad c = \frac{a \sin C_p}{\sin A_p}]^*$$

C_p		Log a	3.616 0983	Log m	
$\frac{\text{Sph. excess}}{3}$		Log b	0.000 5030	Log sin C_p	
C_p	92-42-25	Log tan 45	0.089 29974	Log a	
C_p	46-21-12.5	(45° + ϕ)	0.32 2975	Log b	
$90^\circ - \frac{1}{2}C_p = \frac{1}{2}(A_p + B_p)$	43-28-17.5	ϕ	0.32 2875	Log sph. ex.	
$\frac{1}{2}(A_p - B_p)$	36-14-39.9	Log tan ϕ	0.885 6289	Sph. excess	—
Sum = A_p	79 53 16.4	Log tan $(45^\circ + \phi)$	0.974 4741		
Diff = B_p	07 24 18.6	Log tan $(45^\circ - \phi)$	0.86 1030		
C_p	92 42 25				(Sketch)
	180 0 0				

Log a	3.616 0983
Log sin C_p	9.999 4970
Colog sin A_p	0.006 7912
Log c	3.622 3945



$C = 4191.74$

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.622 3945
1	C			92-42-25.0	0.000 5030
2	B			07 24 18.6	9.110 2024
3	A			79 53 16.4	9.993 2008
1-3				540.88	2.733 0999
1-2				4191.41	3.616 0983

40m Zero Tower

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*The subscripts s and p on this form refer to spherical

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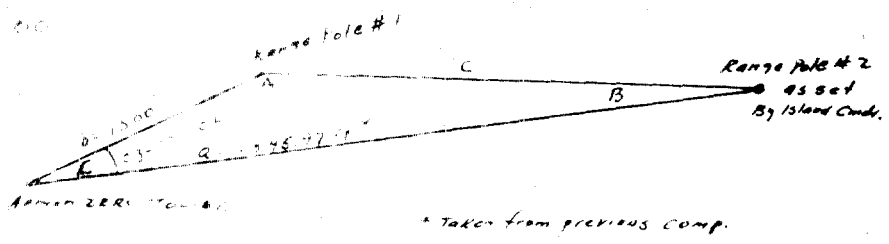
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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$\left[\frac{a}{b} = \tan(45^\circ + \phi) \right]$ (Cal long - side a): $\tan \phi = \frac{a - b}{a + b} \tan \frac{1}{2}(A_p + B_p)$; $c = \frac{a \sin C_p}{\sin A_p}$ (C)

C_s		Log	4.54 2296 ✓	Log m
$\frac{\text{Sph. excess}}{3}$		Log	0.176 0913 ✓	Log sin C_s
C_p	03-17-04.70	Log tan	0.278 1383 ✓	Log a
$\frac{1}{2} C_p$	01 38 32.35 ✓		62' 12" 28.76 ✓	Log b
$90^\circ - \frac{1}{2} C_p = \frac{1}{2}(A_p + B_p)$	88 21 22.65 ✓		17 12 28.76 ✓	Log sph. ex.
$\frac{1}{2}(A_p - B_p)$	84 42 24.76	Log tan	0.490 9475 ✓	Sph. excess —
Sum = A_p	173 04 06.41	Log tan	0.1542 5461 ✓	
Diff = B_p	03 38 42.89	Log tan	0.033 4936 ✓	
C_p	03 17 04.70			Sketch)
	185 00 00.00			
Log a	3.454 2296			
Log sin C_s	8.758 0913			
Colog sin A_p	0.918 0913			
Log c	3.130 7086			



CHECK COMPUTATION

$C = 135.17 \text{ ft}$

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.130 7086
1	C			03 17 04.70	1.241 8730
2	A			173 04 06.41	9.081 6480
3	B			03 38 42.89	8.803 5097
1-3				2845.97	3.454 2296
1-2				1500.00	3.176 0913
2-3					
1					
2					
3					
1-3					
1-2					

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*The subscripts s and p on this form refer to spherical and plane angles respectively.

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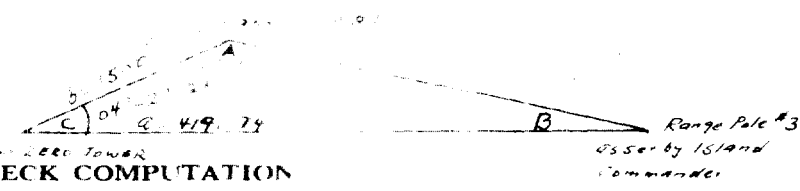
Aomen + Biljiri I

TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

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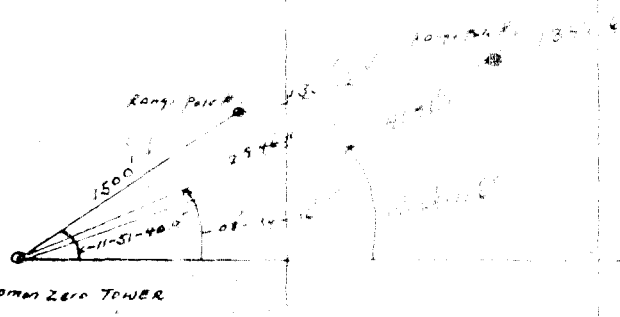
$c = \tan(45^\circ + \phi)$ (Call longer side a) $\tan \frac{1}{2}(A - B) = \frac{a - b}{a + b} \tan \frac{1}{2}(A + B)$ $c = \frac{a \sin C_p}{\sin A_p}$ * (D)

ph. excess				4945	Log m
3				109564	Log sin C_p
04 27 22 3				3032	Log a
02 13 41.15				37"54	Log b
$\frac{1}{2}C_p = \frac{1}{2}(A_p + B_p)$	87	46	18.45	27.54	Log sph. ex.
$A_p - B_p$	85	17	48.10	674 788	Sph. excess
$S_{ph} = A_p$	173	04	07.65	409 9696	
$S_{ph} = B_p$	02	28	30.15	284 257	* From previous comp.
	04	27	22.30		Sketch
	180	00	00.00		
Log a	3.622	3945			
Log sin C_p	8.890	4030			
Colog sin A_p	0.918	3718			
Log c	3.431	1699			



$c = 2698.80 \checkmark$ Ft.

No.	STATION	SIDE/DIVISION ANGLE	SPHERICAL ANGLE	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.431 1699
1	C			04 27 22.30	1.109 5964
2	A			173 04 07.65	9.081 6282
3	B			02 28 30.15	8.635 3248
1-3				419.74	3.622 3945
1-2				1500.0	3.176 0911



*The subscripts a and b on this form refer to spherical and plane angles respectively.

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CHORL A - DISTANCE BASED 1050 from ZERO Tower.

19

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ZERO TOWER	A	$c = 1050$	$a = 564$	Center one of 7 BU IED Cubes
		$b = 1050$		
		$a = 564$		
		$b = 1050$		
		$c = 1050$		
	$2s$	2664		
	s	1332		
	$\log(s-a)$	2.885 362	$\log(s-b)$	2.330 6776
	$\log(s-d)$	2.450 2491	$\log(s-c)$	2.450 2491
	$\log(s-c)$	2.450 2491	$\log(s-a)$	2.885 362
	Σ	7.785 8594	Σ	9.880 4285
	$\log s$	3.124 5042	$\log s$	3.124 5042
		4.661 3552		4.661 3552
	$\log 2$	2.330 6776		2.330 6776
				100-00-000

Aomon ZERO TOWER

* Lieut Smith reports the...
Name has been...
...

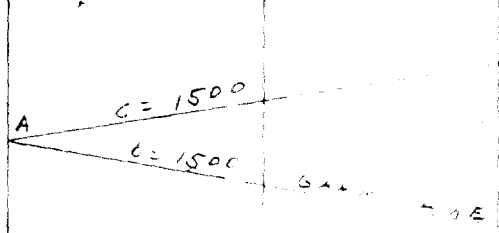
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✓ 14

CHORD A + DIST. COMP. - BU YED - 1500 From ZERO TOWER

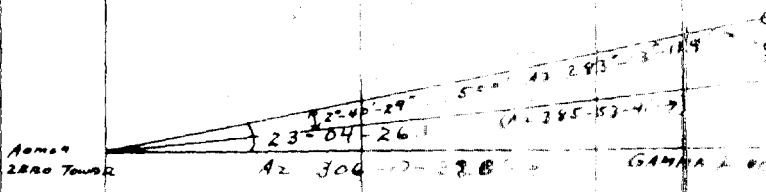
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* Center of Bu YED Cubes

- a = 600
- b = 1500
- c = 1500
- S = 3600
- S = 1800

Log (s-a)	3.079 1812	Log 1.5	2.389 0756	Log 2.0	2.389 0756
Log (s-b)	2.477 1213	Log (s-a)	3.079 1812	Log (s-c)	2.477 1213
Log (s-c)	2.477 1213	Log tan 1/2 A	9.309 8940	Log tan 1/2 B	9.911 9543
Σ	8.033 4238	1/2 A	32 1.504	1/2 B	39 13 53.47
Log s	3.255 2725	A	23 04 26	B	78 27 46.9
	4.778 1513			C	78 27 46.9
Log r	2.389 0756			A	23 04 26.1
					779 59 59.9



BU YED FORMS - (TANG)
 BU YED FORMS - (TANG)
 STATE - BU YED FORMS - (TANG)
 BU YED FORMS - (TANG)
 BU YED FORMS - (TANG)

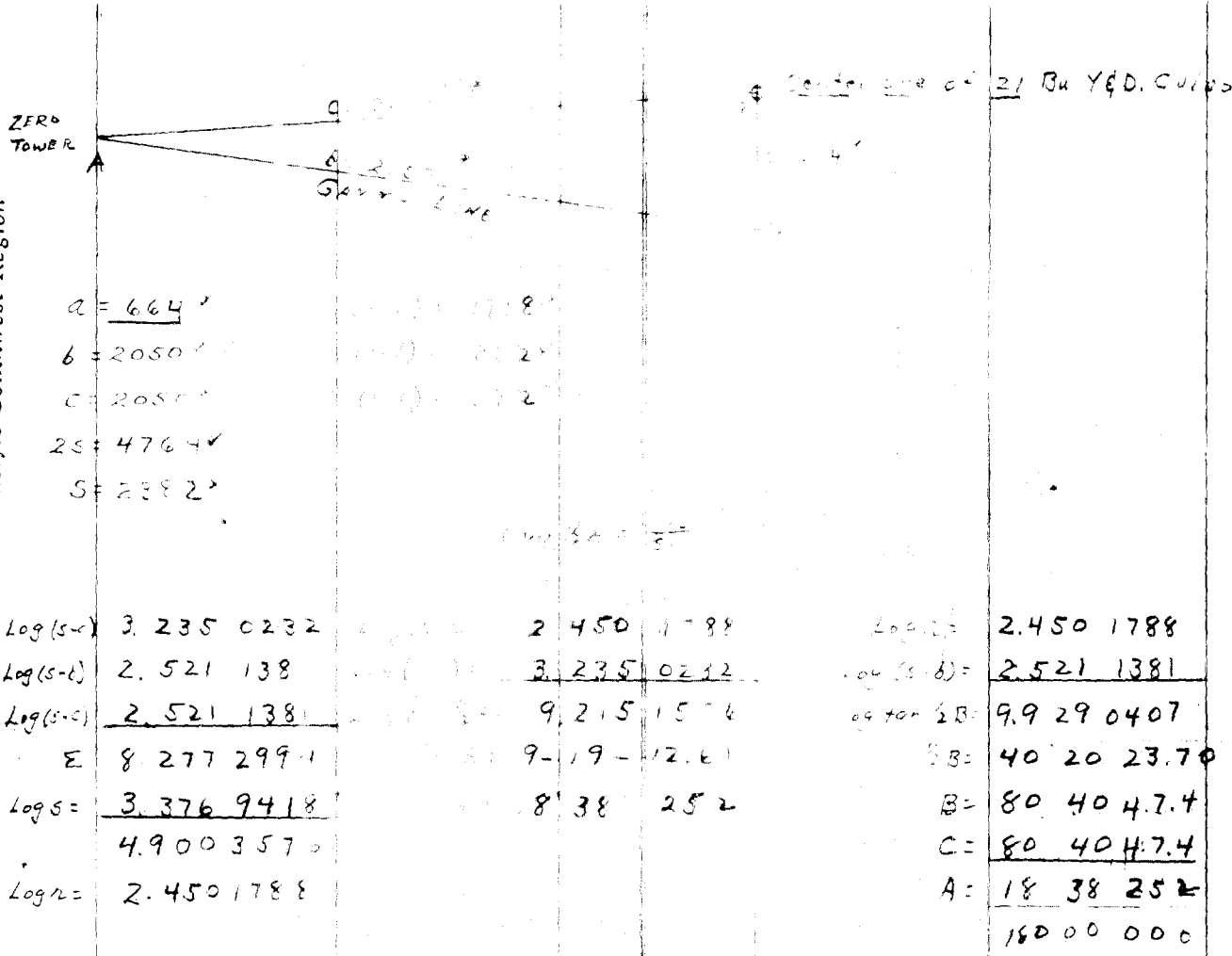
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COMPUTATION OF CHORD & DIST. BU Y&D ~ 2050 FROM ZERO TOWER ✓ 17

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* Note: stake set as computed above fell off high part of Island. stake was set temporarily a dist of 800 set on line to ZERO TOWER giving Radius of 2000 Ft. from ZERO TOWER stake set at this point.

Aoman ZERO TOWER

* Note: Lt Col Erickson CEC USN reports his stake as having been removed or destroyed

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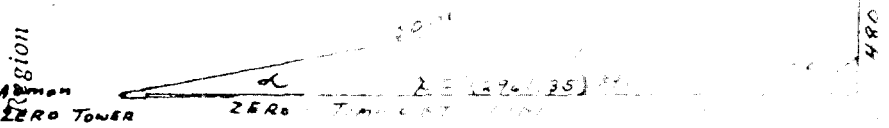
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Comp for Normal offset for ZERO-Tower from 3000' from ZERO Tower

center one of 21 Bo YED units.



$$X^2 = 3000^2 - 480^2$$

$$= 9,000,000 - 230,400 = 8,769,600$$

$$\log X^2 = \log 8,769,600 = 6.9428$$

$$\log 3000 = \log 3,000 = 3.4771$$

$$\log X = \frac{6.9428 - 2 \times 3.4771}{2} = \frac{6.9428 - 6.9542}{2} = \frac{-0.0114}{2} = -0.0057$$

$$\tan \alpha = \frac{480}{296.35}$$

$$\log 480 = 2.6812$$

$$\log 296.35 = 2.4714$$

$$\log \tan \alpha = 2.6812 - 2.4714 = 0.2098$$

$$\alpha = 90^\circ 12' 20''$$

$$\sin \alpha = \frac{480}{3000}$$

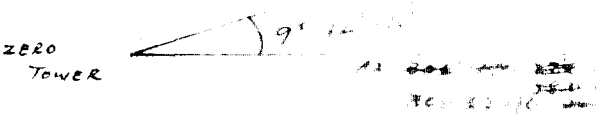
$$\log 480 = 2.6812$$

$$\log 3000 = 3.4771$$

$$\log \sin \alpha = 2.6812 - 3.4771 = -0.7959$$

$$\alpha = 90^\circ 12' 20''$$

292-40
292-00



~~SECRET~~ TRAINING STA. LINE

* Last Code. Enigma... has been removed or destroyed

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*

COMPUTATION OF CNOBL & DIST - Bu 180 - 3600' From ZERO TOWER

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ZERO TOWER	A	Center one of 3 Bu 180 units	
	$c = 360'$		
	$c = 360'$		
$a =$	<u>750'</u>		
$b =$	3600'		
$c =$	3600'		
$2s =$	7950'		
$s =$	3975'		
$\log(s-a)$	3.508 5297	$\log a = 2.528 6276$	$\log c = 2.528 6276$
$\log(s-b)$	2.574 0313	$\log(s-c) = 3.508 5297$	$\log(s-b) = 2.574 0313$
$\log(s-c)$	2.574 0313	$2.574 0313 + 9.020 1314 = 11.594 1627$	$\log \tan \frac{1}{2} B = 9.954 5963$
Σ	8.656 5923	$5.594 1627 - 11.594 1627 = -6.000 0000$	$\frac{1}{2} B = 42 00 37.5$
$\log s =$	3.599 3371	$A = 11 - 57 29.9$	$B = 84 01 15.0$
	5.057 2552		$C = 84 01 15.0$
$\log r =$	2.528 6276		$A = 11 57 29.9$
			179 59 59.9

* Stake for Center one of 3 Bu 180 units



* Last one on map, CEC has reports this stake or hole has been removed or destroyed.

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COMP. OF CHRD & DIST - Bu Y&D - 4050 from ZERO TOWER

30 Bu Y&D Units

	780	(S-0) = 2000			
	4050	(S-6) = 2000			
	4050	(S-0) = 2000			
	8880				
	4440				
Log(S-a)	3.563 4811	Log 2 =	2.549 1136		
Log(S-b)	2.591 0646	Log(S-b)	2.591 0646		
Log(S-c)	2.591 0646	Log(S-c)	9.958 0490		
Σ	8.745 6103		42-14-13.30		
Log 5	3.647 3830		84-28-26.6		
	5.098 2277		84-28-26.6		
Log 2	2.549 1136		11-03-06.7		
			179-59-59.9		

ZERO TOWER

SAMM LINE

STAKE for Center one
of 30 Bu Y&D Units.

* Last Center Stake report...
Stake as follows...

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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$\left[\frac{a}{b} = \tan(45^\circ + \phi) \text{ (Call longer side } a \text{)} \right. \quad \tan \phi = \frac{A - B}{A + B} \quad \phi = \tan^{-1} \left(\frac{A - B}{A + B} \right) \quad c = \frac{a \sin C_p}{\sin A_p} \left. \right]^*$

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C_p		Log	9.8015	Log m
$\frac{\text{Sph. excess}}{3}$		Log	9.8082	Log $\sin C_p$
C_p		Log $\tan \frac{C_p}{2}$	9.8033	Log a
$\frac{1}{2} C_p$		$(45^\circ + \phi)$	21.40	Log b
$90^\circ - \frac{1}{2} C_p = \frac{1}{2}(A_p + B_p)$		ϕ	21.40	Log sph. ex.
$\frac{1}{2}(A_p - B_p)$		Log $\tan \frac{A_p - B_p}{2}$	9.8513	Sph. excess
Sum = A_p		Log $\tan \frac{A_p + B_p}{2}$	9.8768	
Diff = B_p		Log $\tan \frac{A_p - B_p}{2}$	9.2542	
C_p				(Sketch)

Log a	2.7468015
Log $\sin C_p$	9.6603305
Colog $\sin A_p$	0.3936695
Log c	2.8010095

CHECK COMPUTATION

$C = 632.43 \text{ Ft}$

GRAFFLEX $a = (170.144 \text{ m}) \checkmark$
558.215 Ft. \checkmark
Aomon ZERO TOWER

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM	
2-3		<p>NOTE</p> <p>The azimuth shown on this page for the line Aomon Zer - Aomon Photo is in error. The angle Grafflex - Aomon Zer - Aomon Photo should have been added to the azimuth of the line Aomon Zer - Grafflex rather than subtracted. The correct azimuth for the line Aomon Zer - Aomon Photo is 115° (R.I.P.).</p>			2.801 0 095	
1	C				154 45 50.2	0 339 46 15
2	A				22 49 31.84	9 606 33 05
3	B				02 24 31.46	8 774 42 69
1-3					58.215	2 746 80 15
1-2					22.200	1.914 89 79
2-3						

*The subscripts s and p on this form refer to spherical and plane angles respectively.

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U. S. COAST AND GEODETIC SURVEY
Form 665
Ed. Dec. 1929

TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

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$c = \tan(45^\circ + \phi)$ (Call longer side c) $\tan \frac{1}{2}(A_1 - B_1) = \frac{a - b}{a + b} \tan \frac{1}{2}(A_D + B_D)$ $c = \frac{a \sin C_D}{\sin A_D}$ *

		Log c	3.616 0983	Log m	3.616 0983
		Log $\sin C_s$	9.949 1417	Log $\sin C_s$	9.949 1417
	117 05	Log $\tan(45^\circ + \phi)$	2.142 3457	Log a	2.142 3457
	55 30	Log $\tan(45^\circ - \phi)$	1.267 6543	Log b	1.267 6543
		$-\frac{1}{2}C_D = \frac{1}{2}(A_D + B_D)$	3.67	Log sph. ex.	3.67
	27 00	Log $\tan \phi$	1.423 4573	Sph. excess	1.423 4573
	58 30	Log $\tan \frac{1}{2}(A_D + B_D)$	2.657 47		
	04 15	Log $\tan \frac{1}{2}(A_D - B_D)$	1.10 0320		
	117 05	(Sketch)			
		Log a	3.616 0983		
		Log $\sin C_D$	9.949 1417		
		Colog $\sin A_D$	0.068 7260		
		Log c	3.634 3672		
			4308.91		

CHECK COMPUTATION

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.634 3672
1	C				0.050 4577
2	B				8.875 2404
3	A				9.931 2784
1-3				062.12	2.860 0653
1-2				4308.91	3.616 0983
2-3					
1					
2					
3					
1-3				37.8	
1-2					

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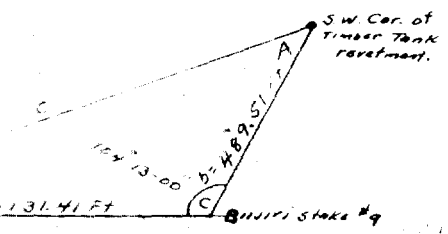
TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$$\left[\frac{a}{b} = \tan(45^\circ - \phi) \quad \text{Calc. } \phi = \frac{1}{2}(A_D - B_D) \quad \text{Calc. } \phi = \frac{1}{2}(A_D + B_D) \quad c = \frac{a \sin C_D}{\sin A_D} \right]^*$$

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C_D	104° 15' 00"	Log	9.816 0983	Log m
$\frac{\text{Sph. excess}}{3}$	0.000 000	Log	2.689 7616	Log sin C_D
C_D	104° 15' 00"	Log	9.926 3367	Log a
$\frac{1}{2} C_D$	52° 07' 30"	Log	14 34.1	Log b
$90^\circ - \frac{1}{2} C_D = -\frac{1}{2}(A_D + B_D)$	37° 48' 00"	Log	14 34.1	Log sph. ex.
$\frac{1}{2}(A_D - B_D)$	21° 11' 23"	Log	9.896 5994	Sph. excess
Sum = A_D	68° 24' 00"	Log	9.891 1165	
Diff = B_D	00° 00' 00"	Log	9.787 7159	
C_D	104° 15' 00"	(Sketch)		
Log c	3.631 2435			
Log sin C_D	9.986 4913			
Colog sin A_D	0.028 4587			
Log c	3.631 2435			

42 802 CHECK COMPUTATION



No.	STATION	HEIGHT	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.631 2435
1					0.013 5087
2					9.971 3461
3					9.045 0105
1-3				4.31.41	3.616 0983
1-2				489.51	2.689 7627
2-3					
1					
2					
3					
1-3					
1-2					

→ GAMMA LINE

*The subscript to a side is on this side for the angles respectively.

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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

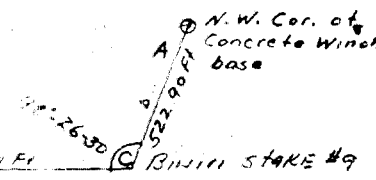
$$\frac{a}{b} = \tan(45^\circ + \phi) \quad (\text{Call longer side } a)$$

$$\tan \frac{1}{2}(A_1 - B_1) = \frac{a - b}{a + b} \tan \frac{1}{2}(A_1 + B_1)$$

$$c = \frac{a \sin C_p}{\sin A_p} \quad *$$

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Log a	3.616 0985	Log b	2.718 4.7	Log m	
Log sin C _p	9.995 2697	Log sin C _e	4.186	Log sin C _e	
Colog sin A _p	0.015 9821	Log tan (45° + φ)	2.794	Log a	
Log c	3.627 3495	(45° + φ)	47.100	Log b	
	4.230 84	Log tan ½(A ₁ - B ₁)	1.005 4762	Log sph. ex.	
		Log tan ½(A ₁ + B ₁)	1.028 7805	Sph. excess	
		Log tan ½(A ₁ - B ₁)	1.028 2533		
			(Sketch)		



CHECK COMPUTATION

No.	STATION	SPHERICAL ANGLE	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3				3.627 3495
1	C			0.104 7309
2	A			9.984 0179
3	B			6.086 3367
1-3			407.41	3.616 0983
1-2			522.90	2.718 4.7
2-3				
1				
2	Home 2580 TOWER			
3				
1-3				
1-2				

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OF THE TRIANGULATION
METHODS AND THE
METHODS FOR

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TRANSVERSE
BASE LINE

COMPUTATION OF *AGENCIER - Agencier Photo Tracer*

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U.S. GEOLOGICAL SURVEY OFFICE 11-2536

SECTION	DATE	DIR. MEAS.	TAPE NO.	TAPE CORR. (FT)	UNCORRECTED LENGTH		TEMP.	TEMP.				ADAPTED LENGTH	(C)	(F)
					Top	Bottom		Top	Bottom	Mean	Max			
1	1913		100											
2	1913		100											
3	1913		100											
4	1913		100											
5	1913		100											
6	1913		100											
7	1913		100											
8	1913		100											
9	1913		100											
10	1913		100											
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35	1913		100											
36	1913		100											
37	1913		100											
38	1913		100											
39	1913		100											
40	1913		100											
41	1913		100											
42	1913		100											
43	1913		100											
44	1913		100											
45	1913		100											
46	1913		100											
47	1913		100											
48	1913		100											
49	1913		100											
50	1913		100											

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U. S. COAST AND GEODETIC SURVEY
FORM 635

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ABSTRACT OF WIRE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

Reproduced from the holdings of the National Archives
Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters	Secs	Meters	Meters	
Bench Δ GRAFLEX						
Bench ADAMON PHOTO TOWER	2.5	0.015	0.2			

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109 12 13

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COMPUTATIONS FOR TRAVERSE BETWEEN
TRIANGULATION STATION GRADLEY AND
ANCHOR POINT POINT

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
FORM 589
Rev. April 1932

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COMPUTATION OF

△ GARFIELD - ADAMS TOWER TRAIL

TRaverse
BASE LINE

Field
Book

U. S. GOVERNMENT PRINTING OFFICE: 1932 (267287-1)

SECTION	DATE	DIR. OF MEAS.	TAPE NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP.	COR.			RECTIONS			REDUCED LENGTH	ADOPTED LENGTH	METERS	(ft.)
					Tape Length	Meters		Temp	Tape and Catenary	Set-up Set-back	Inclination	Sea level	Meters				
1	1922		923		140	140	18.5				+	0.00		140			
2	1922		927		140	140	18.5				+	0.00		140			
3	1922		164		140	140	18.5				+	0.00		140			

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Revised from the findings of the National Academies of Sciences, Engineering, and Medicine. Source: <https://www.nationalacademies.org>

~~SECRET~~
ASTRICAL SURVEY LEVELS
AND

~~OFFICIAL USE ONLY~~

COMPUTATION OF INCLINATION CORRECTIONS

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters	Secs	Meters	Meters	
Mark Δ Graflex						
Bench Δ Graflex						
1						
2						
3						
4						
Bench Aomen Zero Tower						
Mark Aomen Zero Tower						
			Σ			

Reproduced from the holdings of the National Archives
Pacific Southwest Region

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~~SECRET~~

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COMPUTATION OF AVERAGE ...
RCJOA ... AND LISTING ...
STATE # ... RCJOA STATE #

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COMPUTATION OF

AUXILIARY

BASE LINE

For determining distance from Baseline
To Point "O" by triangulation

U. S. GOVERNMENT PRINTING OFFICE: 1935:7334

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SECTION	DATE	DIR. OF MEAS.	T/FF NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP.	COR.			RECTIONS			REDUCED LENGTH	ADOPTED LENGTH	ft.	mm.
					Tape Length	Meters		Temp.	Tape and Tension	Set on Sights	Inclination	Sea level	Meters	Meters	Meters		
10-01-2	1944	A	72	2	44	44.9	18.0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000			
10-01-3	1944	B	73	2	44	44.9	18.0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000			

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 ABSTRACT OF LEVELS
 AND
 COMPUTATION OF INCLINATION CORRECTIONS

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 Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
<i>Raja '0</i>						
<i>BL 1</i>	<i>57</i>	<i>40.706</i>				
<i>BL 2</i>		<i>40.703</i>				
<i>BL 3</i>		<i>40.704</i>				
		<i>5</i>				

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SECRET

Station BIIJIRI II

State Marshall I. (Guam and 401)

Chief of party Ralph L. Egan

Date 7 February 1941

Computed by R. L. P.

Observer G. R. Stode

Instrument H. 200

Checked by R. L. P.

11-9503

Reproduced from the holdings of the National Archives and Records Administration, Pacific Southwest Region

OBSERVED STATION	Observed direction	Centric correction	Station correction	Corrected direction with	Adjusted
				zero initial	direction*
"0"	0 00 00.00			0 00 00.00	
BL 3	315 45 00.00				
BL 2	315 45 00.00				

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1190 1190 1190
 SECRET 1190

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Station ROJOA BL 3

State Marshall Is. (Eniwetok Atoll)

Chief of party Ralph L. Pfanz

Date 7 February 1948

Computed by R. L. P.

Observer G. R. Strode

Instrument H-134

Checked by R. L. P.

11-9503

Reproduced from the holdings of the National Archives
Pacific Southwest Region

OBSERVED STATION	Observed direction	Distance (miles)	Sea level reduction*	Corrected direction with zero initial	Adjusted direction*
ROJOA 'O	0 00 00.00			0 00 00.00	
BIJIRI 'I	67 15 15.7				

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Station RAJOA B L 2

State Marshall I (Guadalupe Atoll)

Chief of party Ralph L. Pfae

Date 7 Feb 1944

Computed by R.L.P.

Observer G.R. Stroe

Instrument T-1

Checked by R.L.P.

Reproduced from the holdings of the National Archives
 Pacific Southwest Region
 RAJOA "0"
 Iijiri 11

OBSERVED STATION	Observed direction	Essential corrections	Final corrected direction	Corrected direction with zero initial	Adjusted direction*
RAJOA "0"	00 00 00			00 00 00	
Iijiri 11	00 00 00				

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Station ROJOA 10

State Marshall Islands (Eniwetok Atoll)

Chief of party Ralph L. ...

Date 1 20 48

Computed by R. L. P.

Observer G. R. Strode

Instrument ...

Checked by ...

Reproduced from the holdings of the National Archives
Pacific Southwest Region

OBSERVED STATION	Observed direction	Observed distance	Sea level reduction	Corrected direction with zero initial	Adjusted direction*
<u>BIJIRII 11</u>	<u>0 00 00.00</u>			<u>0 00 00.00</u>	
<u>ROJOA BL 2</u>	<u>...</u>				
<u>ROJOA BL 3</u>	<u>...</u>				

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 25
Ed. Nov. 1946

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COMPUTATION OF TRIANGLES

Station MARSHALL I. (see p. 16)

Reproduced from the holdings of the National Archives
Pacific Southwest Region

U. S. GOVERNMENT PRINTING OFFICE 16-0210

STATION	PLANE ANGLE	PLAN	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					49 9936 m	2.176 0727 ✓
1 Bluff Hill	31 33 22	100	31 33 22	0	044	0.278 2108
2 Rojoa Blk	21 16 10	100	21 16 10	0	125	9.942 7447
3 Rojoa Blk	27 53 28	100	27 53 28	0	311	9.999 3954
1-3					49 4757 m	2.397 0282
1-2						2.453 6789
2-3					49 9956 m	1.999 9817 ✓
1 Bluff Hill	29 18 13	100	29 18 13	0	249	0.421 9443
2 Rojoa Blk	27 42 52	100	27 42 52	0	562	9.975 0983
3 Rojoa Blk	23 18 36	100	23 18 36	0	389	9.999 3954
1-3					49 4764 m	2.397 0248 ✓
1-2						2.421 3214 ✓
2-3						
1						
2						
3						
1-3						
1-2						
2-3						
1						
2						
3						
1-3						
1-2						

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COORDINATIONS OF AUXILIARY BASE LINE
ACROSS ISLAND AND DISTANCE AROUND STAKE
#10 TO POINT STAKE #1

~~SECRET~~

Page 011
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OFFICIAL USE ONLY COMPUTATION OF

AVULIARI

BASE LINE

(For determination of dist from
Station 2 to Station 7)

U. S. GOVERNMENT PRINTING OFFICE 15-7284

SECTION	DATE	DIR. OF MEAS.	TAPE NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP.	COR.			RECTIONS			REDUCED LENGTH	ADOPTED LENGTH	(ft)	(m)
					Tape Length	Meters		Temp	Tape and Tension	Setup, Curvature	Inclination	Sea level	Meters				
1-2	2-5-35	2	727	2	100	7.5	0.000	0.000	0.000	0.000	0.000	0.000	99.9893	99.9890			
2-3		2	913	2	100	7.5	0.000	0.000	0.000	0.000	0.000	0.000	99.9887	99.9885			
3-4		2	1007	2	100	7.5	0.000	0.000	0.000	0.000	0.000	0.000	99.9887	99.9886			

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Station Aoman 12 State Marshall Is (Linnerton Atoll)
 Chief of party Ralph L. Pfau Date 5 February 1948 Computed by R. L. P.
 Observer G. R. Stode Instrument 7 Receptor Checked by R. L. P.

Reproduced from the holdings of the National Archives
 Region Pacific South

11-0503

OBSERVED STATION	Observed direction	Instrument	Corrected direction with zero initial	Adjusted direction*
Jiri "0"	200 11 00.00		00 00.00	
2	200 11 06.0			
2+25	200 11 15.0			

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Station BL 2

State Marshall Is (Enewetak Atoll)
 (Aomon I)

Chief of party Ralph L Pfau

Date 5 February 1948

Computed by R. L. P.

Observer E. R. Strode

Instrument 7" Repeater

Checked by R. L. P.

Reproduced from the holdings of the National Archives
 Pacific Southwest Region

OBSERVED STATION	Observed direction	Eccentricity correction	Sea level reduction*	Corrected direction with zero initial	Adjusted direction*
<u>Biiijiri C</u>	<u>00 00 00.00</u>			<u>0 00 00.00</u>	
<u>Aomon 12</u>	<u>6 59 12.7</u>				

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U. S. COAST AND GEODETIC SURVEY
Form 24A
Rev. Oct., 1932

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Station BL 2 + 25

State MARSHALL IS. (ENNEWITAK Atoll)
(Aomori I.)

Chief of party Ralph L. Pfau

Date 5 February 1948

Computed by R.L.P.

Observer G.R. Strode

Instrument 7" Repeater

Checked by R.L.P.

11-9503

Reproduced from the holdings of the National Archives and Records Administration, Pacific Southwest Region

OBSERVED STATION

Observed direction

Eccentric reduction

Refraction correction

Corrected direction with zero initial

Adjusted direction*

IRI "0"
omon "12"

0 0 00

0 00 00.00

52 57 12.2

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Station Bijiri O State Marshall Is. (Eniwetok Atoll)
 Chief of party Ralph L. Pratt Date 5 February 1948 Computed by R.L.P.
 Observer G.R. Strode Instrument T. Receptor Checked by R.L.P.

Reproduced from the holdings of the National Archives
 Pacific Southwest Region

OBSERVED STATION	Observed direction	Instrument used	Scale level restriction	Corrected direction with zero initial	Adjusted direction*
<u>Aomon "12"</u>	<u>000 00 00</u>			<u>0 00 00.00</u>	
<u>BL 2</u>	<u>00 00 00</u>				
<u>BL 2 + 25</u>	<u>00 00 00</u>				

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 25
Ed. Nov. 1946

COMPUTATION OF TRIANGLES

State: *Marshall Islands*

U. S. GOVERNMENT PRINTING OFFICE 16-50205-1

Reproduced from the holdings of the National Archives
Pacific Southwest Region

STATION	OBSERVED ANGLE	ADJUSTED ANGLE	LOG. SIN	LOG. COS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					24 4886 m	2 096 8704
1 <i>Bijiri 1</i>	22 12 57.5	100 000			56.5	0.326 7454
2 <i>Aomon 1</i>	45 42 44.4	100 000			45.3	9.993 5996
3 <i>BL 2+25</i>	12 10 10.2	100 000			18.2	9.896 9563
1-3		57.7			00 0	2 417 2154
1-2					209 2050 m	2 320 5721
2-3					99 4896 m	1.999 9522
1 <i>Bijiri 2</i>	21 21 28.5	100 000			52.9	0.398 7540
2 <i>Aomon 12</i>	45 42 51.5	100 000			54.0	9.993 5964
3 <i>BL 2</i>	12 10 22.5	100 000			13.1	9.921 8751
1-3		57.7			00 0	2 392 3026
1-2					209 2095 m	2 320 5813
2-3						
1						
2						
3						
1-3						
1-2						
2-3						
1						
2						
3						
1-3						
1-2						

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DISTANCES AND STAKES OF THE GAINES
STATION LINE TRAVERSE AND OFFSETS
TO ESTABLISH OTHER POINTS

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ABSTRACT OF WYE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

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Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters or feet	Sec	Meters	Meters	
Mark				Σ		
Aoman Zero Tower				17.21		
Bench						
Aoman Zero Tower		+2.124	1.1	0.0		
1		+0.560	1.1	0.3		
2		+0.809	1.2	0.9		
3		+0.024	1.0	0.9		
4		+0.219	1.1	0.9		
5		+0.632	1.1	1.2		
6		+0.066	0.9	1.2		
7		0.220	1.0	1.3		
8		0.373	1.1	1.4		
9		0.410	1.1	1.5		
10		0.234	1.1	1.5		
11		0.912	0.7	1.2		
12		+0.124	1.1	1.3		
13		+0.156	0.9	2.3		
		Σ	2.7			

17 R.Y.P.
1 R.Y.P.

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ABSTRACT OF WYE LEVELS
 AND
 COMPUTATION OF INCLINATION CORRECTIONS

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 Pacific Southwest Region

POINT	DISTANCE <i>Meters</i>	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	<i>Meters</i>				<i>Meters</i>	
<i>Biljiri "0"</i>						
1	170	1.20	0.4			
2	170	1.10	0.3			
3	170	1.00	0.2			
4	170	0.90	0.2			
5	170	0.80	0.2			
6	170	0.70	0.2			
7	170	0.60	0.3			
8	170	0.50	0.4			
9	170	0.40	0.6			
10	170	0.30	0.8			
11	170	0.20	1.2			
			0.7			

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MEAN TEMPERATURES - Gamma Line Traverse

AT THE BENTON POINT

Points Sounding To	From	Area	Temp	Remarks
1-2	34.5	34.5	35.6	To STR #1
2-3	34.4	34.4	34.9	To STR #2
3-4	34.3	34.3	34.0	To STR #4
4-5	34.2	34.2	34.0	To STR #6
5-6	34.1	34.1	34.0	To STR #6
6-7	34.0	34.0	33.8	To STR #8
7-8	33.9	33.9	33.7	To STR #9
8-9	33.8	33.8	33.7	To STR #9
9-10	33.7	33.7	33.3	To STR #11
10-11	33.6	33.6	33.3	To STR #11

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COMPUTATIONS OF THE GAIN STATION
LINE DRAWING

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 ABSTRACT OF WIRE LEVELS
 AND
 COMPUTATION OF INCLINATION CORRECTIONS

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 Pacific Southwest Region

POINT	DISTANCE		MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTIONS	ELEVATION		REMARKS
	Meters	Meters			Meters	Meters	
MARK Aoman Zero Tower Bench Aoman Zero Tower							
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							

~~SECRET~~

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 ABSTRACT
 AND
 COMPUTATION OF INCLINATION CORRECTIONS

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 Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters	Seconds	Meters	Meters	
<i>Buiski 0</i>						
1	50	1.111	1.0			
2	50	1.111	1.0			
3	50	1.111	1.0			
4	50	1.111	1.0			
5	50	1.111	1.0			
6	50	1.111	1.0			
7	50	1.111	1.0			
8	50	1.111	1.0			
9	50	1.111	1.0			
10	50	1.111	1.0			
11	50	1.111	1.0			
			6.0			

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— ON [unclear] —

DATE [unclear]

BY [unclear]

[unclear]

[unclear]

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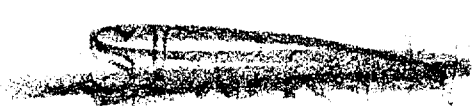
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Location of Structures on Runit Island from Zero Tower

Reproduced from the holdings of the National Archives
Pacific Southwest Region

STRUCTURE	DISTANCE FROM ZERO	AZIMUTH FROM ZERO
Blast Footing	1200	323 08 00
" "	1500	" " "
" "	1800	" " "
" "	2100	" " "
" "	2400	" " "
" "	2700	" " "
" "	3000	" " "
" "	3300	" " "
" "	3600	" " "
" "	3900	" " "
" "	4200	" " "
Blast Building	5250	" " "
Gamma Station A	3150	323 19 31
" " B	3900	" " "
" " C	5400	" " "
Timing Station	2900	323 31 02
Ion. Station	1000	323 00 15
Winch Base	3400	323 08 58
Tank Revetment	5000	324 54 25



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STRUCTURE	DISTANCE FROM ZERO	AZIMUTH FROM ZERO
Photo Tower	43 85 ft.	117 47 20
Triang. Sta. North Base	854 "	110 58 50
Range Pole #1	200 "	316 44 28
" " #2 Dist. from #1	200 "	Az. from #1 321 31 02
" " #3 " " " "	" "	" " " "

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ROBINSON RUNIT ISLAND

STRUCTURE	LATITUDE	LONGITUDE
Triangulation Station North Base	35 21 14.0	182 21 09.890
Triangulation Station Runit	31 16.0	182 22 01.621
Zero Tower	33 20.8	182 21 16.041
Photo Tower	33 28.4	182 21 11.202
Traverse Station Runit	33 44.4	182 21 43.761

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COMPUTATION OF HARTMAN LINE TRAVERSE

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 589
Rev. April 1935

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COMPUTATION OF

TRAVERSE

HARTMAN

~~BASE~~ LINE

(RUN 7 ISLAND)

SECTION	DATE	DIR. OF MEAS.	TAPE NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP. °C	CORRECTIONS					REDUCED LENGTH Meters	ADOPTED LENGTH Meters	Cor.	(mm)
					Tape Length	Meters		Temp	Tape and Catenary	Set-up Set-back	Inclination	Sea level				
Bench RUN 7 ZERO TOWER	1-26-48	F	927	3	4	200	22.8	+0.0017	-0.0101	+0.0176	-0.0027	--	200.0166	200.0166		
" (A)	1-27-48	B	933	3	4	200	22.4	+0.0015	-0.0147	+0.0244	-0.0017	--	200.0105	200.0105		

135

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COMPUTATION OF

TRAVERSE

HARTMAN

BASE LINE

Distance RUNIT ZERO Tower to 4(A) = 200 007.5 m

(RUNIT 134340)

SECTION	DATE	DIR. OF MEAS.	TAPE NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP.	COR.			RECTIONS			REDUCED LENGTH	ADOPTED LENGTH	Dist. FROM RUNIT ZERO T.	NEARS	
					Tape Length	Meters		Temp.	Tape and Catenary	Setup	Inclination	Sea level	Meters					Meters
4(A) To																		
Stake No. 6	127 48	F	921	3	2	100	26.3	10.0000	0.0069	+0.0000	10.0000	---	199.9931	99.99...	300.0000	0006	74 400	
Stake No. 7	127 48	F	921	3	2	100	22.8	10.0000	0.0052	+0.0000	10.0000	---	199.9948	99.99...	300.0000	0006	74 400	
Stake No. 8																		
Stake No. 9																		
Stake No. 10																		
Stake No. 11																		
Stake No. 12																		
Stake No. 13																		
Stake No. 14																		
Stake No. 15																		
Stake No. 16																		
Stake No. 17																		
Stake No. 18																		
Stake No. 19																		
Stake No. 20																		
Stake No. 21																		
Stake No. 22																		
Stake No. 23																		
Stake No. 24																		
Stake No. 25																		
Stake No. 26																		
Stake No. 27																		
Stake No. 28(A)		F	921	3	24	1000	22.8	10.0000	0.0016	+0.0000	10.0000	---	199.9984	99.99...	300.0000	0006	74 400	
Stake No. 28(B)		F	921	3	24	1000	22.8	10.0000	0.0016	+0.0000	10.0000	---	199.9984	99.99...	300.0000	0006	74 400	

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COMPUTATION OF

TRAVERSE

HARTMAN

~~BASE~~ LINE

DISTANCES

RUNIT ZERO TOWER - 4 (A) 200.0075 m
4 (A) to 28 (B) 1199.9485 m
x 1399.9560 v

(RUNIT ISLAND)

U. S. COAST AND GEODETIC SURVEY OFFICE 7524

SECTION	DATE	DIR OF MEAS	TAPE NO	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP		COR			RECTIONS			REDUCED LENGTH	ADOPTED LENGTH	(*) DISTANCE	(**) DISTANCE
					Tape Length	Meters	Temp	Tape and Galvaney	Set-up	Inclination	Sea level	Meters	Meters	Meters				
FROM 28 (B) To:																		
Stake No. 31	11-11-78		92		1	200.0075	28.5	+0.0002	-0.0002	+0.0002	-0.0004			200.0075	199.9485	1199.9485	1399.9560	
Stake No. 32	11-11-78		92		1	200.0075	28.5	+0.0002	-0.0002	+0.0002	-0.0004			200.0075	199.9485	1199.9485	1399.9560	

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based from 100 ft high...
 100 ft high...
 100 ft high...

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ABSTRACT OF WIRE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

(UNIT I)

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POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters Feet	mm	Meters mm	Meters	
28 (B)				2 01 0.15		
29	50	0.59	0.3	3		
30	50	-1.79	3.0	3		
31	50	10.10	0.0	3		
32	50	10.28	0.0	4		
33	50	10.45	0.2	6		
			Σ 0.5			

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 685

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ABSTRACT OF WYE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

RUNIT 15

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POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	<i>Meters</i>	<i>Meters or Feet</i>	<i>sec.</i>	<i>Meters</i>	<i>Meters</i>	
<i>MARK</i>						
<i>RUNIT ZERO TOWER</i>						
<i>BENCH</i>						
<i>RUNIT ZERO TOWER</i>	0	+ 1.92	0			
1	50	+ 1.12	2.7			
2	50	+ 1.12	0.1			
3	50	+ 1.12	2.2			
4 (A)	50	+ 1.12	0.1			
			2.7			

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~~SECRET~~ LEVELS

COMPUTATION OF INCLINATION CORRECTIONS

(RUN 1 I)

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POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	MEAN DIFFERENCE	MEAN ELEVATION	REMARKS
	Meters	Meters	mm	Meters mm	Meters	
4(A)				East cross.		
5	50	+0.04	0.1	0.0		
6	50	-0.62	0.1	0.4		
7	50	+1.04	0.1	1.4		
8	50	+0.44	0.1	1.6		
9	50	+0.54	0.1	1.9		
10	50	-0.04	0.1	1.9		
11	50	-0.40	0.1	2.0		
12	50	-1.16	0.1	3.2		
13	50	-0.16	0.1	3.2		
14	50	-0.10	0.1	3.2		
15	50	+1.38	0.1	5.0		
16	50	+0.79	0.1	5.0		
17	50	-0.58	0.1	5.9		
18	50	+0.50	0.1	6.1		
19	50	+0.95	0.1	6.9		
20	50	+0.42	0.1	7.1		
21	50	-0.18	0.1	7.1		
22	50	-0.54	0.1	7.4		
23	50	-0.74	0.1	7.9		
24	50	-0.32	0.1	8.0		
25	50	-0.40	0.1	8.1		
26	50	-0.92	0.1	8.9		
27	50	+0.32	0.1	9.0		
28(B)	50	-0.34	0.1	9.1		

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MEAN TEMPERATURE COMPUTATION

(RUNIT ISLAND)

HARTMAN LINE TRAVERSE

SECTION 4(A) - 28 (B) FORWARD MEAS

STAKE	Fwd. Therm	Rear Therm	Σ	
4A-5	26.9	26.8	53.7	
5-6	27.0	26.6	53.6	
			10.3	Mean 26.8 To stake No 6
6-7	27.1	26.7	53.8	
			16.1	Mean 26.9 To stake No 7
7-8	27.1	26.8	53.9	
8-9	26.9	26.7	53.6	
			26.2	Mean 26.8 To stake No. 9
9-10	27.1	27.1	54.2	
10-11	26.9	27.1	54.0	
			37.3	Mean 26.9 To stake No. 11
11-12	27.1	27.1	54.2	
12-13	27.1	27.3	54.4	
			48.7	Mean 26.9 To stake No. 13
13-14	27.2	27.1	54.3	
			53.9	Mean 27.1 To stake No. 14
14-15	27.0	27.5	54.5	
			64.9	Mean 27.1 To stake No. 15
15-16	27.1	27.2	54.3	
			64.2	Mean 27.1 To stake No. 16
16-17	27.0	27.1	54.1	
17-18	27.0	27.1	54.1	
			76.5	Mean 27.1 To stake No. 18
18-19	26.9	27.1	54.0	
19-20	27.2	27.1	54.3	
			86.8	Mean 27.1 To stake No. 20
20-21	27.1	27.1	54.2	
21-22	27.0	27.1	54.1	
			97.3	Mean 27.1 To stake No. 22
22-23	27.1	27.1	54.2	
23-24	27.4	27.1	54.5	
			113.5	Mean 27.1 To stake No. 24
24-25	27.7	27.1	54.8	
25-26	27.6	27.1	54.7	
			118.2	Mean 27.1 To stake No. 26
26-27	27.9	27.1	55.0	
27-28(B)	27.4	27.1	54.5	
			309.5	Mean 27.1 To stake No. 28

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MEAN TEMPERATURE COMPUTATION (RUNIT I)

~ HARTMAN LINE TRAVERSE ~

SECTION 28(A) - 33 - FORWARD MEAS.

STAKE	FWD Therm	Rear Therm	Σ
28(A)-29	28.0	8.9	56.9
29-30	27.8	8.5	56.3
30-31	28.0	8.5	56.5
		Σ 269.7	1063 To stake No. 31
31-32	28.8	8.5	58.8
		Σ 228.5	1067 To stake No. 32
32-33	29.6	8.5	60.1
		Σ 288.6	1072 To stake No. 33

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COMPUTATION OF SETUPS, SETBACKS AND OFFSETS
FROM STAKES OF THE HARTMAN LINE TRAVERSE
TO ESTABLISHMENT OF OTHER POINTS

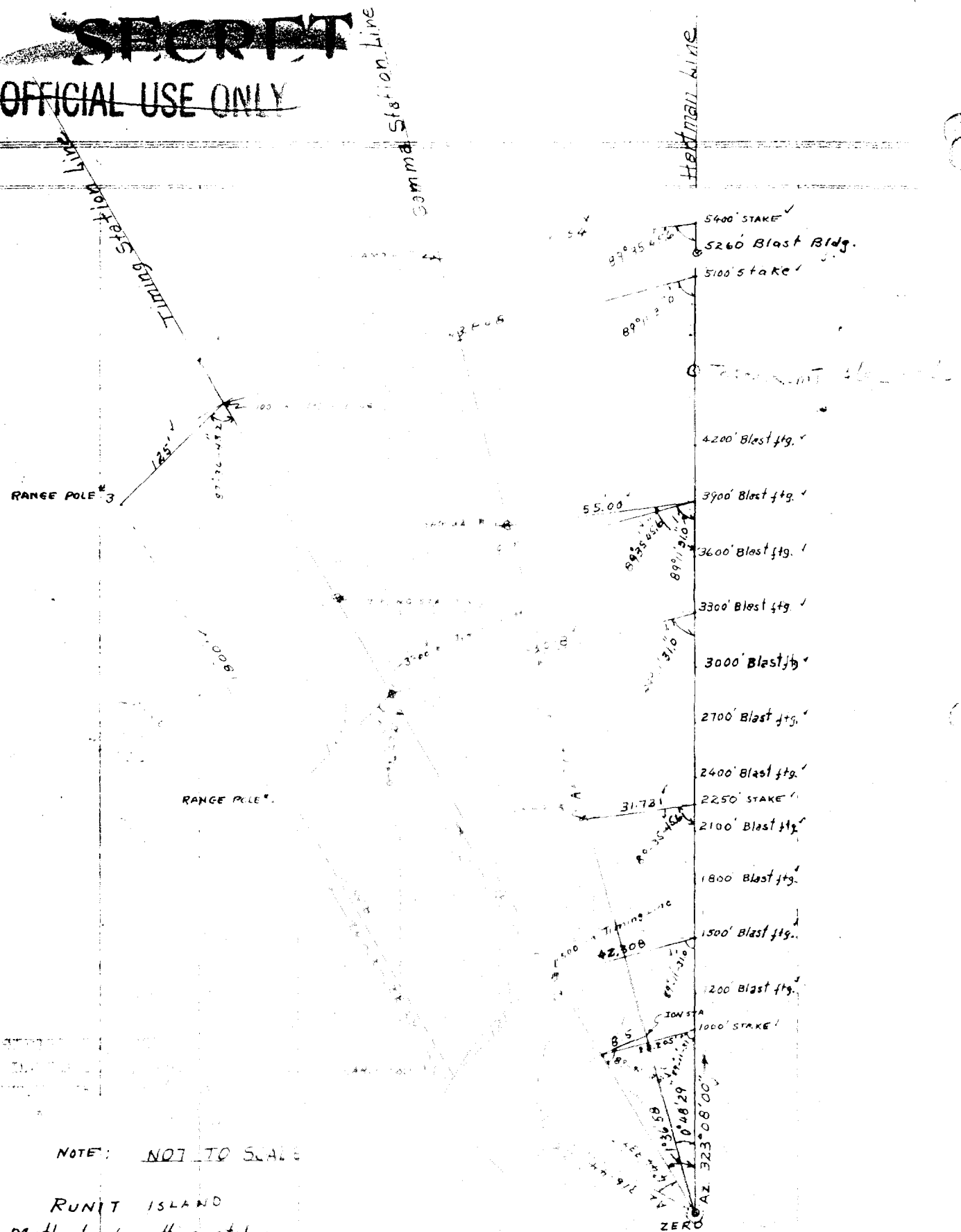
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NOTE: NOT TO SCALE

RUNIT ISLAND
Method of setting stakes
by offset from
HARTMAN TRAVERSE LINE

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RUNIT I

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Computation "Set up" and "Set Backs" etc. of points of the HARTMAN LINE Traverse
To proper distances for establishment of other points by Chord Offsets.

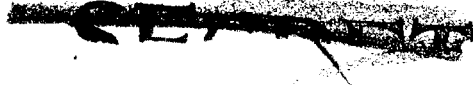
OBJECT TO BE ESTABLISHED	DIST. FROM ZERO TOWER		CLOSEST STAKE TOWER No.	DIST. PER CLOSEST STAKE TOWER	SET UP FROM CLOSEST STK	SET BACK FROM CLOSEST STK	
	Feet	Meters				m	Ft.
Localization Sta.	1000	304.800	6	1000	10748		
Hartman Sta. & Range Pole #1	1500	457.200	7	1500	21574		
Hartman Sta.	1800	548.640	12	1800		2.3679	4.488
Hartman Sta.	2100	640.080	15	2100		9.9199	32.546
Gamma A	2250	685.800	18	2250		14.2225	46.662
Hartman Sta.	2400	731.520	18	2400		18.4973	60.687
Hartman Sta.	2700	822.960	16	2700	25.284		
Hartman Sta.	3000	914.400	15	3000	27.224		
Hartman Sta. & Range Pole #2	3300	1005.840	20	3300	19.231		
Hartman Sta. TIMING STA. GAMMA B	3600	1097.280	22	3600		2.6915	8.830
HARTMAN STA. GAMMA B	3900	1188.720	24	3900		11.2442	36.890
HARTMAN STA. Hartman Sta. & Range Pole #3	4200	1280.160	26	4200		19.7868	64.917
Hartman Sta.	5100	1550.400	27	5100	14.854		
Gamma C	5400	1641.840	30	5400		4.0344	13.236
Hartman Sta.	7200	2194.560	31	7200	01.720		

Blot Bldg - set 10.88 Feet and Hartman Sta. #3 = 5260.0 from zero tower to rear face of Bldg. from original location to the tower by chord offsets.

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CHORD ~~X~~ For offset from HARTMAN LINE 7149.66 to 3900 stake for Timing Station.

A 1936'58"
3900
3900

a = 110

b = 3900

c = 3900

S = 77.0

S = 22.55

RUNIT ZERO TOWER

1900 Timing Sta Stake.

1900 Stake on Hartman Line

s-a = 3845

s-b = 55

s-c = 55

log (s-a) = 3.584 8965

log (s-b) = 1.740 3627

log (s-c) = 1.740 3627

7.065 6217

log S = 3.597 1465

3.468 4750

log 2 = 1.734 2370

log 2 = 1.734 2370

log (s-a) = 3.584 8965

log tan $\frac{1}{2}A$ = 8.49 3417

$\frac{1}{2}A = 0-44-25$

A = 1-88-50

log 2 = 1.734 2370

log (s-b) = 1.740 3627

log tan $\frac{1}{2}B$ = 9.903 8724

$\frac{1}{2}B = 44-51-55$

B-C = 89-11-31

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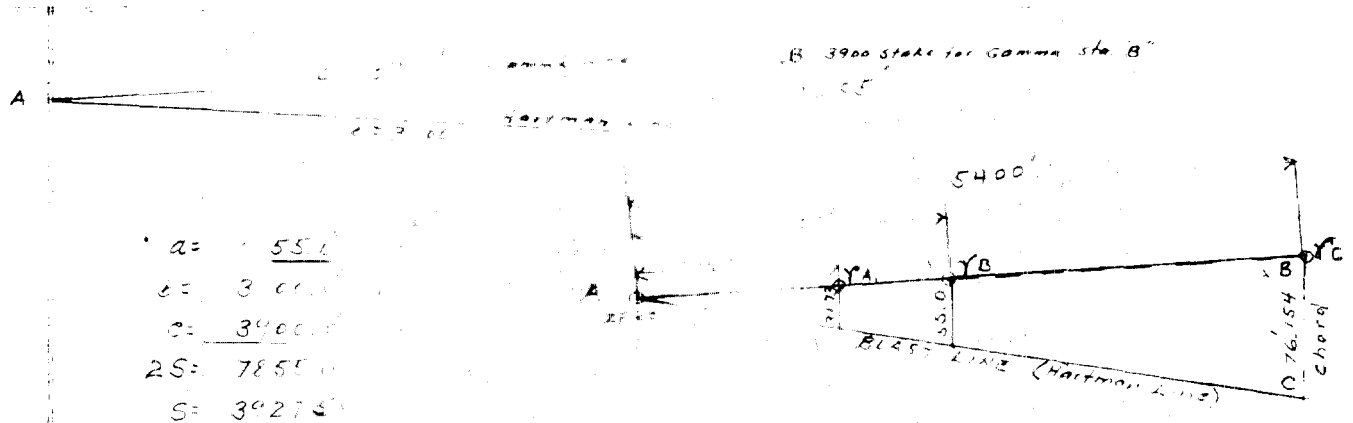
44-A

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RUNIT

CHORD & Distances for offsets from Hartman Line Traverse to GAMMA A, B, & C



$a = 55.1$
 $b = 3000$
 $c = 3400$
 $2S = 7855$
 $S = 3927.5$
 $S-a = 3872.5$
 $S-b = 275$
 $S-c = 275$
 $\log S-a = 3.587$
 $\log S-b = 1.439$
 $\log S-c = 1.439$
 $\Sigma = 6.466$
 $\log S = 3.594$
 2.872
 $\log R = 1.436$
 $\tan \frac{1}{2} A = \frac{R}{S-a}$
 $\log R = 1.436$
 $\log S-a = 3.587$
 $\log \tan \frac{1}{2} A = 7.848$
 $\frac{1}{2} A = 0^\circ - 24 - 14.4$
 $A = 0^\circ - 48 - 28.9$
 $\log R = 1.436$
 $\log S-b = 1.439$
 $\log \tan \frac{1}{2} B = 9.996$
 $\frac{1}{2} B = 44^\circ - 4 - 5.79$
 $B = C = 89 - 35 - 45.6$

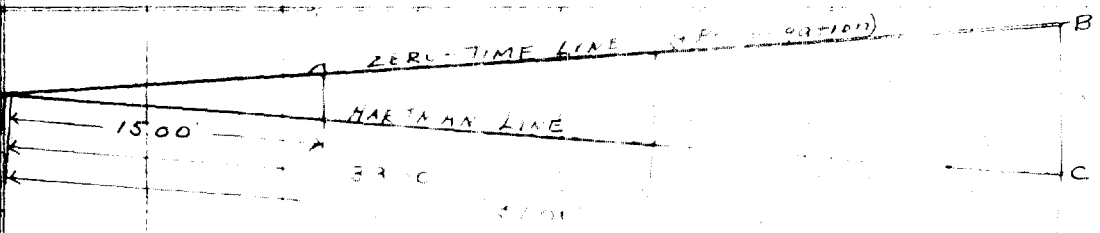
$B = 3900$ Stake for Gamma Sta B
 5400
 76.154 Chord
 $A = 0^\circ - 48 - 28.9$
 $B = 89^\circ - 35 - 45.6$
 $C = 89^\circ - 35 - 45.6$
 $180 - 00 - 00$
 $R = b \sin A = \text{Chord dist} \sim \text{Blast line \& Gamma}$
 $S-a$
 $\log 2750 = 3.352$
 $\log \sin A = 3.149$
 $\Sigma = 1.501$
 $\log \sin B = 9.999$
 $\log \text{dist} = 1.501$
 $\text{dist} = 31.731 \text{ Ft. } \checkmark \text{ GAMMA "A"}$
 $\log 5400 = 3.732$
 $\log \sin A = 8.149$
 $\Sigma = 1.881$
 $\log \sin B = 9.999$
 $\log \text{dist} = 1.881$
 $\text{dist} = 76.154 \text{ Ft. } \checkmark \text{ GAMMA "C"}$

(Dist Comp Checked by similar Δ)

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CHORD OFF SETS FROM HURTMAN LINE TO Z-T LINE (FOR USE IN SETTING RANGE POLES)



- A = 1°-36'-58.0"
- B = 89°-N'-31.0"
- C = 89°-N'-31.0"

From Perce's Computation

$a = \frac{b \sin A}{\sin B}$ = Chord dist. to zero time line

Log 1500 = 3.176 0913	Log 3300 = 3.518 5129	Log 5100 = 3.707 5702
Log sin A = 8.450 2910	Log sin B = 8.450 2910	Log sin A = 8.450 2910
Σ 1.626 3823	Σ 2.068 2219	Σ 2.157 8612
Log sin B = 9.999 9568	Log sin B = 9.999 9568	Log sin B = 9.999 9568
Log dist = 1.626 4275	Log dist = 2.068 2219	Log dist = 2.157 0044
Dist = 42.308 Feet	Dist = 93.078 Feet	Dist = 143.848 Feet

(Distance to perpendicular chord from A)



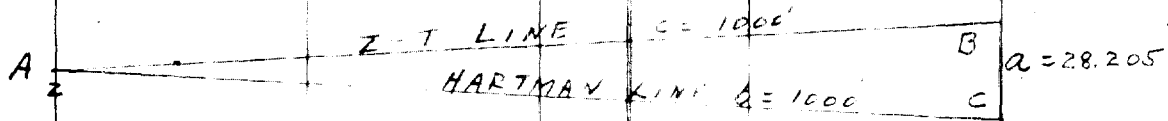
a = 125.0	b = 1500.0	c = 1500.0	2s = 3125.0	s = 1562.5
Log (s-a) = 3.157 6000	Log (s-b) = 1.795 8800	Log (s-c) = 1.795 8800	Σ 6.749 3600	Log s = 3.193 8200
Log r = 1.777 7740	Log (s-d) = 3.157 6000	Log tan 1/2 B = 9.981 8940	Σ 180° 00' 00.1"	Log r = 1.777 7740
	Log r = 1.777 7740	Log r = 1.777 7740		Log (s-b) = 1.795 8800
	Log (s-d) = 3.157 6000	Log tan 1/2 B = 9.981 8940		Log tan 1/2 B = 9.981 8940
	Σ 6.749 3600	Σ 180° 00' 00.1"		B = 43°-48'-21.6"
	Log s = 3.193 8200	A = 4°-46'-33.7"		B = 87°-36'-43.2"
	3.555 5479			C = 87°-36'-43.2"
	Log r = 1.777 7740			A = 04°-46'-33.7"
				Σ 180° 00' 00.1"

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CHORD L + Dist. from 1000 ft. on HARTMAN LINE TRAVERSE To 1000' point on Zero-Timing Station Line

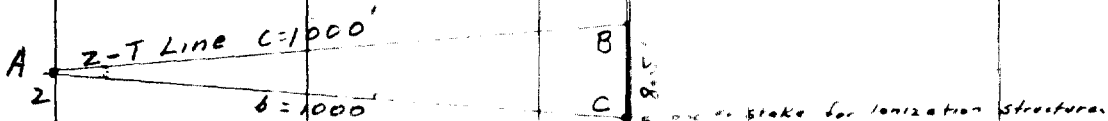


$A = 1^\circ - 36' - 58.0''$
 $B = 89^\circ - 11' - 31.6''$ From previous computation
 $C = 89^\circ - 11' - 31.6''$

$a = \frac{b \sin A}{\sin B}$ Chord dist. Hartman Line to Time Line

$\log 1000 = 3.0000000$
 $\log \sin A = 8.4502910$
 $\Sigma = 1.4502910$
 $\log \sin B = 9.9999568$
 $\log \text{Dist} = 1.4503342$
 $\text{Dist} = 28.205'$ (checked by similar Δ)

CHORD L + Dist From 1000' point on Zero-Timing Station Line to 1000' stake for Ionization Structure.



$a = 8.5$ $s-a = 995.75$
 $b = 1000.0$ $s-b = 4.25$
 $c = 1000.0$ $s-c = 4.25$

$2s = 2008.5$
 $s = 1004.25$

$\log(s-a) = 2.9981503$	$\log = 0.6265432$	$\log r = 0.6265432$
$\log(s-b) = 0.6283889$	$\log(s-a) = 2.9981503$	$\log(s-b) = 0.6283889$
$\log(s-c) = 0.6283889$	$\log \tan \frac{1}{2} A = 7.6283929$	$\log \tan \frac{1}{2} B = 9.9981543$
$\Sigma = 4.254928$	$\frac{1}{2} A = 0^\circ - 14' - 36.63''$	$\frac{1}{2} B = 44^\circ - 52' - 41.72''$
$\log s = 3.001848$	$A = 0^\circ - 29' - 13.3''$	$B = 89^\circ - 45' - 23.4''$
1.2530860		$C = 89^\circ - 45' - 23.4''$
$\log r = 0.6265432$		$A = 0^\circ - 29' - 13.3''$
		$180^\circ 00' 00.1''$

Chord dist is computed to center of face of 7' x 7' structure nearest zero tower

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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$\left[\frac{a}{b} = \tan(45^\circ + \phi) \right]$ (Call longer side a) $\tan \frac{1}{2}(A_D - B_D) = \tan \phi \tan \frac{1}{2}(A_D + B_D)$ $c = \frac{a \sin C_D}{\sin A_D}^*$

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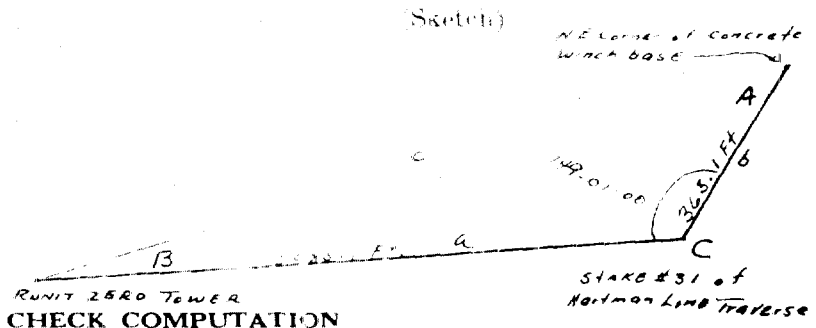
C_D			
Sph. excess			
C_D	149	01	00
C_D	24	30	00
$0^\circ - \frac{1}{2}C_D = \frac{1}{2}(A_D + B_D)$	15	29	00
$(A_D - B_D)$	13	30	82.2
Sum = A_D	28	59	82.2
Diff = B_D	01	59	00
C_D	149	01	00
	180	00	00

Log a	3	706	2995
Log b	2	560	0262
Log $\tan \frac{1}{2}(A_D - B_D)$	8	539	2733
Log $\tan \frac{1}{2}(A_D + B_D)$	8	539	5668
ϕ	44	54	5668
Log $\tan \frac{1}{2}A$	4	927	8729
Log $\tan \frac{1}{2}B$	1	412	7430
Log $\tan \frac{1}{2}C$	1	410	6159

Log m	
Log $\sin C_D$	
Log a	
Log b	
Log sph. ex.	
Sph. excess	0.0

(Sketch)

Log a	3	706	2995
Log $\sin C_D$	9	711	6290
Colog $\sin A_D$	0	314	4354
Log c	3	732	3639
$C = 5399.65$ FT			



No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					3.732 3639
1	C			149 01 00	0.289 3710
2	B			01 59 01.74	8.539 2920
3	A			28 59 58.26	9.685 5647
1-3				63.1	2.560 0269
1-2				5085.1	3.706 2996

2-3					
1	ZERO TOWER			NE corner of concrete winch base	
2					
3					
1-3					
1-2					

*The subscripts a and b on this form refer to the sides of the triangle respectively.

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COMPUTATION FOR DETERMINING DISTANCE AND AZIMUTH
BETWEEN ZERO TOWER AND PHOTO TOWER

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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

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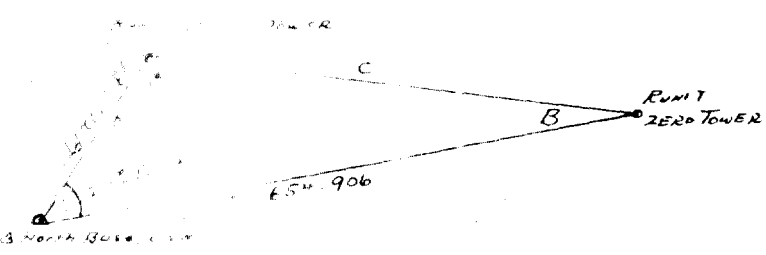
$$\left[\frac{a}{b} = \tan(45^\circ + \phi) \quad (\text{Call longer side } a) \quad \tan \phi = \frac{a - b}{a + b} \quad \tan \phi \tan \frac{1}{2}(A_D + B_D) \quad c = \frac{a \sin C_D}{\sin A_D} \right]^*$$

C_D		Log a	2.735 4514	Log m	
Sph. excess		Log b	0.310 4841	Log sin C_D	
$\frac{3}{3}$		Log tan 45°	0.000 0000	Log a	
C_D	29-1-247	($\frac{1}{2} \phi$)	0.000 0000	Log b	
$90^\circ - \frac{1}{2} C_D = \frac{1}{2}(A_D + B_D)$	85-2-2953	$\frac{1}{2} C_D$	0.000 0000	Log sph. ex.	
$(A_D + B_D)$	68-4-2953	Log tan $\frac{1}{2} C_D$	0.000 0000	Sph. excess	
Sum = A_D	143-54-0557	Log tan $\frac{1}{2}(A_D + B_D)$	0.000 0000		
Diff = B_D	66-4-2953	Log tan $\frac{1}{2}(A_D - B_D)$	0.000 0000		
C_D	29-1-247				(Sketch)

Log a	2.816 1790
Log sin C_D	9.689 5577
Colog sin A_D	0.229 7560
Log c	2.735 4514

C = 543.82 ft

CHECK COMPUTATION



No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3					2.735 4514
1	C			29-1-247	0.310 4841
2	A			143-54-0557	9.770 2435
3	B			66-4-2953	9.073 8891
1-3				654.906	2.816 1790
1-2				2.222	2.119 8246

2-3					
1					
2					
3					
1-3					
1-2					

*The subscripted angles in this form refer to opposite and included angles respectively

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145A

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COMPUTATION OF TRAVERSE BETWEEN TRIANGULATION
STATION NORTH BASE AND PHONE TOWER

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 699
Rev. April 1964

~~SECRET~~ COMPUTATION OF ~~BASE LINE~~ ^{TELEGRAPH} BASE LINE

SECTION	DATE	DIR MEAS	TAPE NO	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP	COR			RELATIONS			ADAPTED LENGTH	ADJUSTED LENGTH	COR	
					Tape length	Meters		Level	Tape and reference	Sag and back	Inclination	See level	Meters				Meters
1	1944	F	72	3	1000	1000	15	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
2	1944	F	74	3	1000	1000	15	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
3	1944	F	76	3	1000	1000	15	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

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ALTIMETER AND WYR LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

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POINT	DISTANCE		MEAN DIFFERENCE OF ELEVATION	INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters feet		min	Meters	Meters	
MARK N. Base, U.S.N.							
Bench N. Base, U.S.N.	0		1.348	0			
1	50		-0.317	0			
Bench RUNIT Photo tower	483		0.002	0			
MARK RUNIT Photo Tower	0		3.98	0			
				Σ			

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COMPUTATION OF TRAVERSE BETWEEN TRIANGULATION
STATION NORTH BASE AND ZERO TOWER

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COMPUTATION OF ^{TRAVERSE} ~~BASE~~ LINE
N Base. W.N. - Run 1 ZERO Tape

U. S. GOVERNMENT PRINTING OFFICE 11-7200 (Run 1)

SECTION	DATE	DIR. OF MEAS.	TAPE NO.	TAPE SUPPORT	UNCORRECTED LENGTH		TEMP	COR		RECTIONS			REDUCED LENGTH	ADOPTED LENGTH	(ft)	(mm)
					Tape Loops	Meters		Temp	Tape and Catenary	Set-up Set-back	Inclination	Sea level				
							Meters	Meters	Meters	Meters	Meters	Meters	Meters			
<i>Beach</i>																
<i>N Base. W.N. Run</i>	<i>2-20-58</i>	<i>E</i>	<i>921</i>	<i>3</i>	<i>4</i>	<i>200</i>	<i>28.1</i>	<i>+0.0007</i>	<i>-0.0101</i>	<i>+0.0203</i>	<i>0.0014</i>		<i>199.6155</i>	<i>199.6156</i>		
<i>Beach</i>		<i>E</i>	<i>3441</i>	<i>2</i>			<i>28.4</i>	<i>+0.0000</i>	<i>+0.0000</i>	<i>-0.3340</i>						
<i>Beach</i>																
<i>Run 1 ZERO Tape</i>																

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COMPUTATION FOR DETERMINING TANK REVETMENT LOCATION

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ABSTRACT OF WYE LEVELS
AND
COMPUTATION OF INCLINATION CORRECTIONS

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Pacific Southwest Region

POINT	DISTANCE	MEAN DIFFERENCE OF ELEVATION		INCLINATION CORRECTION	ELEVATION	MEAN ELEVATION	REMARKS
	Meters	Meters	or feet	mm	Meters	Meters	
28 (B)							
1	50	-0.25		0.0			
2	50	-1.60		2.4			
3	50	-0.35		0			
4	50	-0.60		0.3			
5	50	+0.22		0.0			
				2.8			

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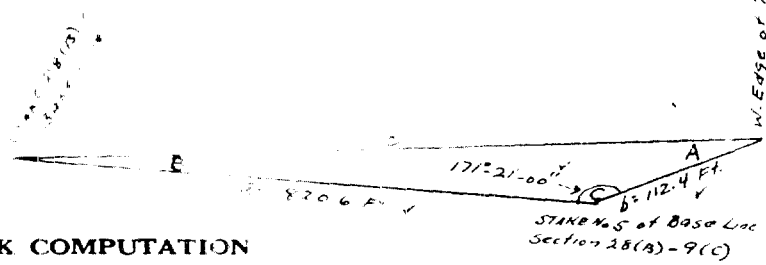
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TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

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$$\left[\frac{a}{b} = \tan(45^\circ + \phi) \quad (\text{Call longer side } c) \quad \tan \frac{1}{2}(A_p - B_p) = \tan \phi \tan \frac{1}{2}(A_p + B_p) \quad c = \frac{a \sin C_p}{\sin A_p} \right]^*$$

C_p			Log a	4 4 1315	Log m
ph. excess			Log b	9 7 7663	Log $\sin C_p$
C_p	171 21 00		Log $\tan 45^\circ$	9 6 3 3652	Log a
$0^\circ - \frac{1}{2}C_p = \frac{1}{2}(A_p + B_p)$	85 40 30		$(45^\circ + \phi)$	2 02 03	Log b
$(A_p - B_p)$	04 19 30			12 02 03	Log sph. ex.
Sum = A_p	03 17 04 00		Log $\tan \phi$	4 880 2743	Sph. excess
Diff = B_p	07 36 28 03		Log $\tan \frac{1}{2}(A_p + B_p)$	4 274 6895	
C_p	01 22 2 2 00		Log $\tan \frac{1}{2}(A_p - B_p)$	4 258 9638	
	<u>171 21 00</u>				(Sketch)
	180 00 00 00				
Log a	2.914 1315				
Log $\sin C_p$	9.177 2425				
Colog $\sin A_p$	0.877 9806				
Log c	2.969 3576				



$c = 931.88 \text{ Ft.}$

CHECK COMPUTATION

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
	2-3			931.9 Ft	2.969 3576
1	C			171 21 00	0.822 7575
2	B			01 02 21.97	8.258 6513
3	A			07 36 38.03	9.122 0164
	1-3				2.050 7664
	1-2				2.914 1315
	2-3				
1					
2					
3					
	1-3				
	1-2				

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*The subscripts s and p of this form refer to spherical and plane angles respectively.

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DEPARTMENT OF COMMERCE
U. S. COAST AND GEODETIC SURVEY
Form 665
Ed. Dec. 1929

TRIANGLE COMPUTATION USING TWO SIDES AND INCLUDED ANGLE

$$\left[\frac{a}{b} = \tan(45^\circ + \phi) \quad (\text{C.L. on the side } a); \quad \tan \phi = \frac{B_p}{A_p} = \tan \phi \tan \frac{1}{2}(A_p + B_p); \quad c = \frac{a \sin C_p}{\sin A_p} \right]^*$$

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C_p	70	3	42.3	Log	9.229	0964	Log m
$\frac{\text{Sph. excess}}{3}$				Log	2.969	3576*	Log $\sin C_p$
C_p	70	3	42.3	Log tan	0.652	7388*	Log a
$\frac{1}{2} C_p$	35	1.5	21.15	Log tan	0.27	28.62	Log b
$90^\circ - \frac{1}{2} C_p = \frac{1}{2}(A_p + B_p)$	54	1.5	58.85	Log tan	0.27	28.62	Log sph. ex.
$\frac{1}{2}(A_p + B_p)$	53	1.5	43.38	Log tan	0.78	3 4837	Sph. excess
Sum = A_p	5	5.5	52.2	Log tan	0.91	8655	
Diff = B_p	0	7.0	25.49	Log tan	8.73	5 3504	
C_p	70	3	42.3			3492	

(Sketch) * Taken from previous comp.

No.	STATION	SPHERICAL ANGLE	SPHERICAL EXCESS	PLANE ANGLE AND DISTANCE	LOGARITHM
2-3				5109.7 Ft.	3.708 3927 ✓
1	C			170 13 42.3	0.770 2651 ✓
2	A			01 59 52.21	9.143 4386 ✓
3	B			01 46 25.49	8.490 6997 ✓
1-3					3.622 0964
1-2					2.969 3575

*The subscripts 2 and 3 in this form refer to spherical angles respectively.

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